

GENERAL NOTES

- THE WORK SHOWN ON THESE DRAWINGS HAS BEEN DESIGNED IN ACCORDANCE WITH THE STRUCTURAL REQUIREMENTS OF THE 2003 BUILDING CODE OF NEW YORK STATE (IBC 2018 - AMENDMENTS) AND THE 2003 EXISTING BUILDING CODE OF NEW YORK STATE.
- THE STRUCTURAL COMPONENTS HAVE BEEN DESIGNED FOR THE FOLLOWING LOADS:

A FLOOR LOAD LOAD:	
RETAIL:	100 PSF
DINING ROOMS AND RESTAURANT:	100 PSF
TOILET ROOMS:	80 PSF
B ROOF SNOW LOAD:	
GROUND SNOW LOAD (Pg):	50 PSF
FLAT ROOF SNOW LOAD (Pf):	36.5 PSF
2 SNOW EXPOSURE FACTOR (Ce):	1.0
3 SNOW IMPORTANCE FACTOR (Is):	1.0
4 THERMAL FACTOR (Ct):	1.1
C ROOF LIVE LOAD:	20 PSF
D WIND DESIGN DATA:	
1 BASIC WIND SPEED (3-SECOND GUST):	115 MPH
2 RISK CATEGORY: II:	
3 WIND EXPOSURE:	B
4 INTERNAL PRESSURE COEFFICIENT:	0.18
5 COMPONENTS AND CLADDING(A + 50 SF):	
WALLS: WITHIN 8 FT OF CORNER:	+30.4 PSF
OTHER:	-30.2 PSF
WALLS: OTHER:	+30.4 PSF
WITHIN 8 FT OF EAVE:	-30.5 PSF
ROOF: OTHER:	+19.4 PSF
WITHIN 8 FT OF CORNER:	+40.4 PSF
OTHER:	+19.4 PSF
ROOF: OTHER:	-29.9 PSF

- E EARTHQUAKE DESIGN DATA:
- SEISMIC IMPORTANCE FACTOR (IE): 1.00
 - MAPPED SPECTRAL RESPONSE: 0.230g
 - WIND EXPOSURE: 0.054g
 - SITE CLASS: S1
 - SPECTRAL RESPONSE COEFFICIENTS: 0.10g
 - SEISMIC DESIGN CATEGORY: B
 - BASIC SEISMIC FORCE-RESISTING SYSTEM: LIGHT FRAMED WALLS w WOOD SHEAR PANELS, ORDINARY PLAN MASONRY SHEAR WALLS
 - DESIGN BASE SHEAR: 0.120 * W
 - SEISMIC RESPONSE COEFFICIENT (CS): 0.120
 - RESPONSE MODIFICATION FACTOR (R): 1.5
 - ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE PROCEDURE
 - INTERIOR PARTITION: 5 PSF
 - OTHER LOADS: CONCENTRATED LOADS ON ALL FLOORS (ON 2-10 FEET SQUARE): 2,000 LBS OR AS INDICATED IN PLAN

- ALL STRUCTURAL WORK SHOWN OR SPECIFIED ON THESE DRAWINGS IS SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER OF RECORD. ASPECTS OF THE WORK FOUND TO BE DEFECTIVE BECAUSE IT DOES NOT MEET THE REQUIREMENTS SHOWN OR SPECIFIED SHALL BE CORRECTED BY THE CONTRACTOR AT NO EXTRA COST TO THE OWNER AS DIRECTED BY THE ENGINEER.
- THIS WORK HAS BEEN DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE CONSTRUCTION HAS BEEN COMPLETED. THE STABILITY OF THE STRUCTURE PRIOR TO COMPLETION IS SOLELY THE RESPONSIBILITY OF THE CONTRACTOR. THIS RESPONSIBILITY EXTENDS TO ALL ASPECTS OF THE CONSTRUCTION ACTIVITY INCLUDING, BUT NOT LIMITED TO, JOBSITE SAFETY, ERECTION METHODS, ERECTION SEQUENCE, TEMPORARY BRACING AND SHORING, USE OF EQUIPMENT AND SIMILAR CONSTRUCTION PROCEDURES. REVIEW OF CONSTRUCTION BY THE ENGINEER IS FOR CONFORMANCE WITH THE DESIGN ASPECTS ONLY. NOT TO REVIEW THE CONTRACTOR'S CONSTRUCTION PROCEDURES. LACK OF COMMENT ON THE PART OF THE ENGINEER WITH REGARD TO CONSTRUCTION PROCEDURES IS NOT TO BE INTERPRETED AS APPROVAL OF THOSE PROCEDURES.

- FOUR COPIES OF SHOP DRAWINGS, ERECTION AND DETAIL DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR THROUGH THE ARCHITECT TO THE ENGINEER FOR REVIEW FOR ALL REINFORCING BARS, AND WOOD TRUSSES. SHOP DRAWINGS SHALL INDICATE THE FABRICATOR, MANUFACTURER, LAYOUT, MATERIALS, FINISH, AND ACCESSORIES, AND SHALL BE CHECKED BY THE CONTRACTOR AND BEAR THE CHECKER'S INITIALS BEFORE SUBMISSION TO THE ARCHITECT AND ENGINEER. STRUCTURAL DRAWINGS WILL NOT BE USED AS SHOP DRAWINGS.
- THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS, ANGLES AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS AND EXISTING CONDITIONS PRIOR TO PROCEEDING WITH ANY WORK.
- ALL SECTIONS AND DETAILS SHALL BE CONSIDERED TYPICAL AND APPLIED FOR THE SAME AND SIMILAR SITUATIONS THROUGHOUT THE BUILDING, UNLESS OTHERWISE SPECIFICALLY NOTED.
- IF FAULTY CONSTRUCTION PROCEDURES, OR MATERIAL, RESULT IN DEFECTIVE WORK THAT REQUIRES ADDITIONAL ENGINEERING TIME TO DEVISE CORRECTIVE MEASURES, PROFESSIONAL FEES MAY BE CHARGED TO THE CONTRACTOR AT THE STANDARD HOURLY RATE OF ADDITIONAL SERVICES. SUCH FEES MAY BE WITHHELD FROM THE CONTRACTOR'S PAYMENT.
- DO NOT SCALE DRAWINGS.

- FOUNDATION AND EXCAVATION NOTES:
- THE FOUNDATIONS HAVE BEEN DESIGNED TO REST ON NATIVE SOIL OR CONTROLLED FILL HAVING A PRESUMPTIVE BEARING VALUE OF 2,000 PSF EXPECTED TO BE FOUND AT THE BOTTOM OF THE REQUIRED EXCAVATION TO BE VERIFIED BY A GEOTECHNICAL ENGINEER PRIOR TO CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED IF SOIL OF QUESTIONABLE CAPACITY IS ENCOUNTERED DURING EXCAVATION.
 - WITHIN THE PERIMETER OF THE PROPOSED STRUCTURE STRIP THE GROUND SURFACE OF ALL TOPSOIL, ORGANIC AND FILL MATERIAL, THEN COMPACT THE TOP OF THE REMAINING SURFACE. THE EXTENT OF REMOVAL SHALL EXTEND 5'-0" MIN. BEYOND THE BUILDING PERIMETER. PLACE MINOR SAND OR EQUIVALENT GEOTEXTILE FABRIC OVER THE COMPACTED SUBGRADE FOLLOWED BY 1'-0" OF CRUSHED STONE (AN EQUAL BLEND OF NYSDOT No. 1 AND No.2; ANOTHER LAYER OF GEOTEXTILE STABILIZATION FABRIC AND EITHER LIFTS OF ADDITIONAL CRUSHED STONE OR STRUCTURAL FILL (PER THE GEOTECHNICAL REPORT) UP TO THE SUBGRADE ELEVATION FOR FOUNDATIONS AND SLABS. SEE GEOTECHNICAL REPORT FOR APPROXIMATE DEPTHS OF EXCAVATION REQUIRED.
 - THE BOTTOM OF EXTERIOR FOOTINGS NOT ON SOLID ROCK SHALL BE AT LEAST 4'-0" BELOW FINISHED GRADE. THE SURFACE OF THE SOIL BELOW ALL FOOTINGS SHALL BE MECHANICALLY COMPACTED PRIOR TO SETTING FOOTING FORMS. FOOTINGS ON EDGE SHALL REST ON BROOK/CLEAN SOLID ROCK. IF THE SLOPE OF THE ROCK SURFACE EXCEEDS 1:1 AND THE FOOTING SHALL BE DOWELED TO THE EDGE WITH 3/4" STEEL RODS DRILLED 10 INCHES INTO THE ROCK SURFACE AT 2 FEET ON CENTER.
 - PROTECT ALL SOIL UNDER FOUNDATIONS FROM FREEZING DURING CONSTRUCTION. DO NOT POUR CONCRETE ON FROZEN SOIL.
 - IF STANDING WATER IS PRESENT IN THE FOOTING EXCAVATION 4 TO 6 INCH THICK LAYER OF 3/4" CRUSHED STONE SHALL BE COMPACTED INTO THE BOTTOM OF THE EXCAVATION AND WATERING METHODS USED THAT WILL NOT UNDERMINE THE BEARING OF THE NEW FOOTINGS.
 - DO NOT UNDERMINE EXISTING OR NEWLY PLACED FOUNDATIONS BY EXCAVATING WITHIN A ZONE DIRECTLY BELOW THESE FOUNDATIONS AND EXTENDING DOWN AND OUTWARDS AT A 1 ON 1 SLOPE.
 - IN AREAS REQUIRING FILL, THE FILL MATERIAL SHALL BE A UNIFORMLY GRADED MIXTURE OF SAND AND GRAVEL WEIGHING NO LESS THAN 120 PPF DRY DENSITY AFTER COMPACTION IN PLACE. THIS MIXTURE SHALL BE UNIFORMLY GRADED HAVING NO STONE GREATER THAN 4 INCHES IN ANY ONE DIMENSION, 30-75% PASSING A 1/4" SIEVE, 5-40% PASSING A No. 40 SIEVE, AND WITH LESS THAN 10% PASSING A #200 SIEVE (ALL PERCENTAGES BY WEIGHT). THE FILL SHALL BE PLACED IN MAXIMUM LIFTS OF 8 INCHES BEFORE COMPACTION. EACH LIFT SHALL BE COMPACTED WITH APPROPRIATE EQUIPMENT TO A MINIMUM OF 98% OF ITS MAXIMUM DENSITY AT OR NEAR OPTIMUM MOISTURE. AS DETERMINED BY MODIFIED PROCTOR TESTS. A SOILS TESTING LAB, HIRED BY THE OWNER, SHALL TEST THE MATERIAL BEFORE AND AFTER COMPACTION FOR CONFORMANCE WITH THIS SPECIFICATION. NO LIFTS SHALL BE PLACED WHEN WEATHER CONDITIONS ARE SUCH THAT THE MOISTURE CONTENT OF THE FILL CANNOT BE PROPERLY CONTROLLED.
 - IN PLACING AND COMPACTING FILL AND BACKFILL MATERIAL, DO NOT DAMAGE NOR DISPLACE CONCRETE WORK ALREADY IN PLACE BY CONTACT FROM COMPACTION MACHINERY, BY SUBJECTING IT TO OVERTURNING FROM HEAVY COMPACTION LOADINGS, OR ANY OTHER CAUSE. AT FROST WALLS BRING FILL AGAINST SUCH CONCRETE AT THE SAME RATE AS THE REMAINDER OF FILL, COMPACTING UNIFORMLY ON BOTH SIDES USING HAND OPERATED TAMPERS. IN BASEMENT CRAWL SPACE AREAS DO NOT BACKFILL AGAINST WALLS UNTIL THE FLOOR OR ROOF DECK BEARING ON THE WALLS HAS BEEN INSTALLED AND FULLY ATTACHED TO THE TOP OF THE FOUNDATION.
 - MINIMUM ANCHOR BOLT REQUIREMENTS FOR ATTACHMENT OF WOOD SUPERSTRUCTURE TO FOUNDATION SHALL BE AS FOLLOWS:
 - 5/8" DIAMETER AT 4'-0" o.c. MAX SPACING, AND WITHIN 1'-0" OF WALL ENDS
 - EMBED ANCHOR BOLTS A MINIMUM OF 7 INCHES INTO NEW CAST CONCRETE. INSTALL BOLTS WITHIN 12 INCHES OF CORNERS ON ALL EXTERIOR WALLS. ALL SILL PIECES SHALL HAVE A MINIMUM OF TWO ANCHOR BOLTS.
 - IF ANCHOR RODS ARE NOT CAST, DRILL AND EMBED 7" MIN w/ HILTI "HIT HY 200"
 - SIZES AND LOCATIONS OF ALL REQUIRED EMBEDDED ITEMS FOR ALL TRADES SUCH AS ANCHOR BOLTS, PIPING SLEEVES, HOLD-DOWN ANCHORS, ETC., SHALL BE COORDINATED BY THE GENERAL CONTRACTOR WITH OTHER TRADES.
 - INVERTS OF FOOTING DRAIN, IF REQUIRED, ARE TO BE SET A MINIMUM OF 2 INCHES ABOVE THE BOTTOM OF THE ADJACENT FOOTINGS.

CONCRETE NOTES:

- STRUCTURAL CONCRETE WORK SHALL CONFORM TO ALL THE REQUIREMENTS OF A.C.I. 318-14 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" IN ITS ENTIRETY. CERTAIN PORTIONS OF THIS SPECIFICATION ARE PRESENTED HERE ONLY FOR CLARIFICATION AND THE CONTRACTOR'S CONVEENIENCE AND ARE NOT INTENDED TO REPLACE OR AMEND THIS SPECIFICATION.
- CONCRETE SHALL BE NORMAL WEIGHT EXCEPT AS NOTED BELOW, DEVELOP A MINIMUM 28 DAY STRENGTH, F.C. AND HAVE A MAXIMUM WATER/CEMENTITIOUS MATERIAL RATIO, (W/C+P), AS FOLLOWS:

LOCATION	F.C.	W/C+P
FOOTINGS, FOUNDATIONS	3000 PSI	0.50
INTERIOR SLABS ON GRADE	3000 PSI	0.45
EXTERIOR SLABS ON GRADE	4000 PSI	0.42
- NO ADMIXTURES ARE PERMITTED WITHOUT THE ENGINEER'S WRITTEN PERMISSION. CONCRETE EXPOSED TO THE WEATHER, SUCH AS THAT USED IN FOUNDATION WALLS SHALL CONTAIN 6% ± 1.5% ENTRAINED AIR.
- CEMENT SHALL BE TYPE I OR TYPE II AND CONFORM TO ASTM C 150.
- OTHER CEMENTITIOUS MATERIAL SUCH AS FLYASH OR GROUND GRANULATED BLAST-FURNACE SLAG MAY BE BLENDED WITH CEMENT FOR USE IN THE CONCRETE MIX. FLYASH SHALL CONFORM TO ASTM C618 AND MAY REPLACE CEMENT IN THE FOLLOWING RANGES FOR THE 2 CLASSES OF FLYASH, CLASS C, 20 TO 30%, CLASS F, 10 TO 20%. GROUND GRANULATED BLAST-FURNACE SLAG SHALL CONFORM TO ASTM C698 AND MAY NOT EXCEED 50% OF TOTAL WEIGHT OF CEMENTITIOUS MATERIALS.
- COARSE AGGREGATE SHALL BE 3/4" AND CONFORM TO ASTM C 33.
- REINFORCING STEEL SHALL CONFORM TO ASTM A 615, GRADE 60.
- NO WELDING OF REINFORCING WILL BE PERMITTED.
- CONCRETE FORMWORK SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 8, ACI 318 - 14.
- FABRICATION AND PLACEMENT OF REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 7, ACI 318 - 14.
- THE PRODUCTION OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 5 ACI 318 - 14.
- THE CONVEYANCE, PLACEMENT AND PROTECTION OF THE CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 5, ACI 308 - 14, SECTIONS 5.1 THROUGH 5.10. MECHANICAL VIBRATORS ARE TO BE USED TO CONSOLIDATE THE FRESHLY CAST CONCRETE AROUND THE REINFORCING AND AGAINST FORM SURFACES AND TO PREVENT THE FORMATION OF AIR OR STONE POCKETS, HONEYCOMBING, PITTING OR PLANES OF WEAKNESS. HOWEVER, CARE MUST BE USED TO AVOID OVERVIBRATION THAT CAN LEAD TO AGGREGATE SEGREGATION.
- THE INSTALLATION OF SLABS SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 11, ACI 301 - 14. INTERIOR FINISH SLAB SURFACES ARE TO HAVE A CLASS A STEEL TROWEL FINISH. SURFACES OF SLABS FORMING THE SUBSTRATE FOR MUDJBS ARE TO HAVE A CLASS C SCRATCHED SURFACE. EXTERIOR SLAB SURFACES ARE TO HAVE A CLASS B TOLERANCE WITH THE FINISH AS SPECIFIED ON THE ARCHITECTURAL DRAWINGS.
- THE CURING AND PROTECTION OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 5 ACI 318 - 14, SECTIONS 5.11. CONCRETE SLABS SHALL BE PROTECTED FROM LOSS OF SURFACE MOISTURE FOR NOT LESS THAN 7 DAYS USING A CURING COMPOUND CONFORMING TO ASTM C 309 OR CONSTANTLY WETTED BURLAP. IF COLD WEATHER CONCRETING CONDITIONS EXIST AS DEFINED BY A PERIOD OF MORE THAN THREE DAYS WHEN THE AVERAGE OUTDOOR TEMPERATURE, (HIGH + LOW)/2, IS LESS THAN 40 F., THE PROCEDURES OUTLINED IN ACI 308R-16 STANDARD SPECIFICATION FOR "COLD WEATHER CONCRETING" SHALL BE UTILIZED. CURING COMPOUNDS SHALL BE COMPATIBLE WITH ANY INTENDED FLOORING OVERLAY. DO NOT INSTALL FINISH FLOORING UNTIL SLABS HAS ADEQUATELY CURED PER THE FLOORING MANUFACTURER'S SPECIFICATIONS.
- THE FOLLOWING SUBMITTALS ARE TO BE MADE TO AND APPROVED BY PRESTON ENGINEERING PRIOR TO COMMENCING ANY WORK:
 - (1) CONCRETE DESIGN MIX FOR EACH STRENGTH OF CONCRETE REQUIRED ATTESTING THAT THE MIXES CAN ATTAIN THE MINIMUM REQUIRED STRENGTHS IN ACCORDANCE WITH CHAPTER 5 OF ACI 318-14.
 - (2) CONCRETE ENGINEER REINFORCING FIBERS SHALL BE POLYPROPYLENE, COLLATED, FIBRILLATED FIBERS FROM FIBERMESH COMPANY, 4019 INDUSTRY DRIVE CHATHAM/KNICK, TN OR EQUAL, APPROVED BY THE ENGINEER. FIBERS SHALL BE USED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS BUT IN NO INSTANCE WITH A DOSAGE RATE OF LESS THAN 1.12 LB. OF FIBERS PER CUBIC YARD OF CONCRETE.
- A DESIGNATED TESTING LABORATORY SHALL CONDUCT STRENGTH TEST IN ACCORDANCE WITH THE FOLLOWING PROCEDURES:
 - (A) STRENGTH TEST CONSISTS OF FOUR CONCRETE CYLINDERS (3).
 - (A) MAKE ONE STRENGTH TEST FOR EACH 50 CUBIC YARDS OR FRACTION THEREOF FROM EACH MIX DESIGN OF CONCRETE PLACED IN ANY ONE DAY, EXCEPT THAT IN NO CASE SHALL A GIVEN MIX DESIGN BE REPRESENTED BY LESS THAN FIVE TESTS.
 - (B) SECURE COMPOSITE SAMPLES IN ACCORDANCE WITH "METHOD OF SAMPLING FRESH CONCRETE" (ASTM C 172). EACH STRENGTH TEST SHALL BE OBTAINED FROM A DIFFERENT BATCH OF CONCRETE USING REPRESENTATIVE, TRULY RANDOM BASIS. WHEN PUMPING OR PNEUMATIC EQUIPMENT IS USED, SAMPLES SHALL BE TAKEN AT THE DISCHARGE END.
 - (C) MOLD FOUR SPECIMENS FROM EACH SAMPLE IN ACCORDANCE WITH "METHOD OF MAKING AND CURING CONCRETE COMPRESSION AND FLEXURE SPECIMENS IN THE FIELD" (ASTM C 21), AND CURE UNDER STANDARD MOISTURE AND TEMPERATURE CONDITIONS, IN ACCORDANCE WITH SECTION 7(A) AND 7(B) OF THE ABOVE ASTM METHOD.
 - (D) MINIMUM SLUMP OF THE CONCRETE SAMPLE FOR EACH STRENGTH TEST AND WHENEVER CONSISTENCY OF CONCRETE APPEARS TO VARY USING "METHOD OF TEST OF SLUMP OF PORTLAND CEMENT CONCRETE" (ASTM C 143).
 - (E) DETERMINE AIR CONTENT OF NORMAL WEIGHT CONCRETE SAMPLE FOR EACH STRENGTH TEST IN ACCORDANCE WITH EITHER "METHOD OF TEST FOR AIR CONTENT OF FRESHLY MIXED CONCRETE BY PRESSURE METHOD" (ASTM C 201), "METHOD OF TEST FOR AIR CONTENT OF FRESHLY MIXED CONCRETE BY THE VOLUMETRIC METHOD" (ASTM C 173).
 - (F) TEST THREE SPECIMENS: ONE AT SEVEN DAYS, AND TWO AT 28 DAYS IN ACCORDANCE WITH "METHOD OF TEST FOR COMPRESSIVE STRENGTH OF MOLDED CONCRETE CYLINDERS" (ASTM C 39). THE 28 DAY TEST RESULT SHALL BE THE AVERAGE OF THE TWO SPECIMENS. IF THE AVERAGE OF THE TWO SPECIMENS IS LESS THAN THE REQUIRED STRENGTH, TEST THE FOURTH SPECIMEN AT 45 DAYS. WHEN HIGH EARLY STRENGTH IS REQUIRED, TWO SPECIMENS SHALL BE TESTED AT SEVEN DAYS.

STRUCTURAL STEEL NOTES:

- DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO AISC 360-10 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION.
- MATERIALS:
 - WIDE FLANGE(W) SHAPES: ASTM A 992
 - STANDARD (S), MISCELLANEOUS (M), ANGLES (L), PILES(HP): A572, GR 50
 - PLATES AND BARS: A36
 - HOLLOW STRUCTURAL SECTIONS (HSS)- SQUARE, RECTANGULAR & ROUND: ASTM A 500, GRADE B
 - PIPE (P): ASTM A 53, TYPE S, GRADE B
 - BOLTS: ASTM A325
 - ANCHOR RODS: ASTM F 1554
 - WELDING ELECTRODE: ASTM E70XX - LOW HYDROGEN
- ALL WELDING SHALL CONFORM TO THE CODE FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION OF THE AMERICAN WELDING SOCIETY AND BE PERFORMED BY A CERTIFIED WELDER IN ACCORDANCE WITH A W.S. STANDARD. ALL WELDS ARE TO BE CLEANED OF SLAG TO PERMIT VISUAL INSPECTION.
- FOR ALL STEEL CONNECTIONS NOT INDICATED IN THESE DRAWINGS, FABRICATOR SHALL DESIGN AND FABRICATE EACH CONNECTION FOR A MINIMUM SHEAR REACTION OF 10 KIPS.
- CERTIFICATES OF COMPLIANCE ARE REQUIRED TO BE SUBMITTED TO THE ENGINEER FOR STRUCTURAL STEEL, BOLTS, NUTS, WASHERS AND WELD FILLER MATERIAL PRIOR TO THE FABRICATION OF ANY STEEL.
- FOR MISCELLANEOUS STEEL, SEE ARCHITECTURAL DRAWINGS.
- ALL STEEL MEMBERS AND BOLTING EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A 123. MINIMUM ACCEPTABLE ZINC COATING WEIGHT SHALL BE 2.02 PER SQ. FT. SEE ARCH. DRAWINGS FOR FINISHED PAINT.
- ALL FIELD WELDING IS TO BE VISUAL INSPECTED BY AN A.W.S. CERTIFIED WELD INSPECTOR. REPORTS ARE TO BE SENT TO THE ARCHITECT, ENGINEER, AND OWNER IN A TIMELY MANNER.
- FASTEN WOOD NAILER TO TOP FLANGE OF STEEL BEAM w/ (2) ROWS 12" Ø THROUGH BOLTS AT 16" o.c. ON BEAM GAGE.

GENERAL WOOD NOTES:

- WOOD DESIGN IS BASED ON THE ANSII/AWC NDS-2018 "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION".
- STUD BEARING WALLS, SHEARWALLS, AND ROOF AND FLOOR DECK SHALL BE FRAMED WITH THE MEMBER SIZES AND/OR TYPES AT THE SPACINGS SHOWN ON PLAN. THE CONTRACTOR SHALL COORDINATE LOCATIONS OF ALL PLUMBING PIPING, HVAC DUCTING AND RECESSED LIGHTING FIXTURES, ETC PRIOR TO LAYOUT TO MINIMIZE INTERFERENCE THAT MAY REQUIRE THE ALTERNING OR STRENGTHENING OF THE INSTALLED FRAMING.
- ALL WOOD FRAMING IS TO BE STORED ON SITE ABOVE THE GROUND ON "STICKERS" INDOORS OR UNDER TARPS WITH ADEQUATE CLEARANCES TO ALLOW AIR CIRCULATION.
- WALLS SHALL BE INSTALLED STRAIGHT AND PLUMB. FLOORS SHALL BE INSTALLED LEVEL AT THE PROPER ELEVATION. ROOFS SHALL BE INSTALLED AT THE PITCHES INDICATED ON THE ARCHITECTURAL DRAWINGS.
- JOISTS AND RAFTERS SHALL BE INSTALLED DIRECTLY OVER BEARING STUDS UNLESS OTHERWISE DETAILED.
- JOISTS SHALL BE SUPPORTED LATERALLY AT EACH SUPPORT AND AT MIDSPAN BY FULL DEPTH SOLID BLOCKING TWO INCHES IN THICKNESS, EXCEPT WHERE JOISTS ARE SUPPORTED BY A FLUSH HEADER OR NAILED TO A RM JOIST.
- PROVIDE A MINIMUM OF TWO STUDS AT EACH END OF ALL FLUSH FRAMED HEADERS OR BEAM. UNLESS MORE ARE INDICATED ON PLAN. PROVIDE ONE JACK STUD AND ONE FULL KING STUD AT EACH END OF ALL DROPPED HEADERS OR BEAMS, UNLESS MORE JACK AND KING STUDS ARE INDICATED ON PLAN.
- FLUSH FRAMED CONNECTIONS SHALL BE MADE WITH PREFABRICATED GALVANIZED STEEL HANGERS MADE BY SIMPSON STRONG-TIE. CO., INC. OR KANT-SAG CONNECTIONS BY UNITED STEEL PRODUCTS CO. OF WIDTH AND DEPTH APPROPRIATE FOR THE SUPPORTED MEMBER. INSTALL WITH THE TYPE AND QUANTITY OF FASTENERS RECOMMENDED BY THE MANUFACTURER. PREFABRICATED STEEL HANGERS USED IN CONTACT WITH PRESERVATIVE PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED, STAINLESS STEEL, OR HAVE A "TRIPLE ZNC" (ASTM G185) COATING. FASTENERS IN CONTACT WITH PRESERVATIVE PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL. DO NOT MIX STAINLESS STEEL WITH GALVANIZED FASTENERS OR CONNECTORS.
- STRUCTURAL WOOD FRAMING USED IN EXTERIOR APPLICATIONS OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE SOUTHERN YELLOW PINE NO. 2 OR BETTER, ACQ (ALKALINE COPPER QUARTERNARY) OR CA (COPPER AZOLE) PRESERVATIVE PRESSURE TREATED WOOD WITH A RETENTION APPROPRIATE FOR THE END USE.
- ENDS OF WOOD GIRDER ENTERING AN EXTERIOR CONCRETE OR MASONRY WALL SHALL HAVE A MINIMUM 1/2" AIR SPACE ON ITS TOP, END AND SIDES UNLESS IT IS MADE FROM NATURALLY DURABLE OR PRESERVATIVE TREATED WOOD.
- BUILT-UP MEMBERS OF THREE PLYS OR LESS SHALL HAVE ADJACENT PLYS NAILED TOGETHER WITH TWO ROWS OF NAILS AT 12" O.C. (100 COMMON NAILS FOR 1-1/2" PLYS, 120 COMMON NAILS FOR 1-3/4" PLYS); BUILT-UP MEMBERS OF MORE THAN 3 PLYS SHALL BE ASSEMBLED WITH 1/2" DIAMETER TRUSS BOLTS AT 16" O.C. STAGGERED UP AND DOWN WITH 2 INCH CLEARANCE AT TOP AND BOTTOM EDGES.
- FULTCH BEAMS ARE TO BE ASSEMBLED WITH 1/2" DIAMETER TRUSS BOLTS AT 16" O.C., STAGGERED UP AND DOWN, WITH 2 INCH CLEARANCE AT TOP AND BOTTOM EDGES. ASSEMBLE FULTCH BEAM WITH BOTTOMS OF WOOD PLYS AND STEEL PLATE FLUSH. PROVIDE 3-1/2" X 3/8" X 3-1/2" STEEL BEARING PLATES AT ENDS OF FULTCH BEAMS.
- EXTERIOR END WALLS OF CATHEDRAL CEILING SPACES SHALL BE FRAMED WITH STUDS RUNNING CONTINUOUSLY (NOT SPLICED) FROM FLOOR TO ROOF.
- JOISTS AND BEAMS SHALL NOT BE LESS THAN 1 3/4" MIN EACH END FOR SINGLE SPAN BEAMS AND 3 1/2" MIN OVER INTERIOR SUPPORTS (I.E. FOR TWO-SPAN BEAMS)

DIMENSIONED WOOD FRAMING NOTES:

- THE STRUCTURAL WOOD STRESS GRADE STAMPED LUMBER SHALL BE GRADED AS FOLLOWS:
 - RAFTERS AND HEADERS: DOUGLAS FIR-LARCH OR DOUGLAS FIR-LARCH (NORTH) OR SPRUCE-PINE-FIR (NO. 2 OR BETTER)
 - FB (BASE) = 850 PSI
 - E = 1,400,000 PSI
- STUDS: SPRUCE-PINE-FIR, STUD GRADE
- FB (BASE) = 875 PSI
- E = 1,200,000 PSI
- THE DESIGN OF THE DIMENSIONAL LUMBER MEMBERS AND THEIR CONNECTIONS IS BASED ON THE LUMBER HAVING A MOISTURE CONTENT AT THE TIME OF INSTALLATION OF 19% OR LESS.
- JOISTS OR RAFTERS ARE TO BE INSTALLED WITH "CROWN" UP (I.E. POSITIVE CAMBER) AND WITHIN 1/2 INCH OF STRAIGHT, END-TO-END ALIGNMENT.
- SEVERELY DISTORTED OR DAMAGED LUMBER (TWISTED, BOWED, CUPPED, SPLIT, CHECKED, ETC.) SHALL NOT BE USED.
- NOTCHES IN THE TOP OR BOTTOM OF DIMENSIONED LUMBER JOISTS OR RAFTERS SHALL NOT EXCEED ONE-SIXTH THE MEMBER DEPTH AND SHALL NOT BE LOCATED IN THE MIDDLE THIRD OF THE SPAN. END NOTCHES SHALL NOT EXCEED ONE-FOURTH THE MEMBER DEPTH. BORED HOLES SHALL NOT BE WITHIN TWO INCHES OF THE TOP AND BOTTOM OF THE MEMBER AND THEIR DIAMETER SHALL NOT EXCEED ONE-THIRD THE MEMBER DEPTH.
- LAMINATED VENEER LUMBER (LVL), LAMINATED STRUCTURAL LUMBER (LSL) AND PARALLEL STRAND LUMBER (PSL) NOTES
 - 1 LAMINATED VENEER LUMBER SHALL BE "MICRO LAM" AS MANUFACTURED BY TRUS JOIST MACMILLAN, "G-P LAM" AS MANUFACTURED BY THE GEORGIA PACIFIC CORPORATION OR "SANG LAM" AS MANUFACTURED BY THE LOUISIANA PACIFIC CORPORATION. LAMINATED STRUCTURAL LUMBER SHALL "POWER BEAM" AS MANUFACTURED BY ANTHONY FOREST PRODUCTS COMPANY. PARALLEL STRAND LUMBER SHALL BE "PARALLAM" AS MANUFACTURED BY TRUS JOIST MACMILLAN.
 - 2 MINIMUM ALLOWABLE STRESS AND STIFFNESS CHARACTERISTICS SHALL BE AS FOLLOWS:
 - FB = 2600 PSI
 - FC = 2510 PSI (PARALLEL TO GRAIN)
 - FE = 750 PSI (PERPENDICULAR TO GRAIN)
 - FX = 285 PSI
 - E = 1,900,000 PSI
 - ANY OF THESE MATERIALS MAY BE USED INTERCHANGEABLY FOR MEMBERS THAT ARE NOTED ON PLAN SINGULARLY AS LVL. MEMBER SIZES SHOWN ON PLAN (WIDTH X DEPTH) MAY BE CONSTRUCTED OF MULTIPLE PLYS OF THE SPECIFIED DEPTH, FASTENED TOGETHER BY NAILING OR BOLTING AS REQUIRED. MEMBER SIZES FOLLOWED ONLY BY PSL MUST BE INSTALLED AS A SOLID MEMBER, NOT BUILT-UP.
 - MEMBERS MAY NOT BE BORED OR NOTCHED WITHOUT WRITTEN PERMISSION FROM THE ENGINEER.

SHEATHING PANELS

- SHEATHING PANELS, PLYWOOD OR ORIENTED STRAND BOARD (OSB), SHALL CONFORM TO U.S. PRODUCT STANDARD PS-1 AND BEAR THE APA GRADE/TRADEMARK OF THE AMERICAN PLYWOOD ASSOCIATION.
- SHEATHING PANELS FOR WALLS SHALL BE APA RATED SHEATHING, EXPOSURE 1, WITH A MINIMUM SPAN INDEX RATING OF 2016. PANELS SHALL BE 1/2" THICK.
- SHEATHING PANELS ON FLAT SURFACES SHALL BE INSTALLED WITH FACE GRAIN PERPENDICULAR ACROSS TO SUPPORTS AND CONTINUOUS OVER TWO OR MORE SPANS.
- WALL SHEATHING SHALL BE FASTENED TO METAL STUDS AT 4" o.c. AT ALL EDGES AND AT 6" o.c. MAX AT ALL INTERIOR SUPPORT POINTS

WOOD FASTENERS NOTES:

- WOOD COMPONENTS ARE TO BE FASTENED TOGETHER AS INDICATED IN THE FOLLOWING SCHEDULE UNLESS SPECIFICALLY INDICATED OTHERWISE ON THE PLANS.

BUILDING ELEMENT	NAIL SIZE/TYPE	NUMBER AND LOCATION
FLOOR CONSTRUCTION		
BUILT UP GIRDER/ BEAM (3 PLYS OR LESS)	10D FOR 1.5" PLYS 16D FOR 1.75" PLYS	2 HORIZONTAL ROWS (TOP & BOTTOM) 12" O.C. DIRECT
FLOOR JOISTS TO SILL OR GIRDER:	16D COMMON	4 TOE-NAIL
EDGE FLOOR JOIST/ TRUSS TO SILL:	16D COMMON	TOE-NAIL 8" O.C.
BRIDGING TO JOISTS:	16D COMMON	2 DIRECT 3 TOE-NAIL
RIMBO JOIST TO A JOIST END:	16D COMMON	3 END DIRECT
FLOOR TRUSS CHORD TO SILL OR GIRDER:	16D COMMON	3 DIRECT
BAND JOIST TO TRUSS END:	16D COMMON	3 DIRECT
EDGE FLOOR TRUSS BOTTOM CHORD TO SILL:	16D COMMON	8" O.C. DIRECT
WALL CONSTRUCTION		
STUD TO SOLE PLATE:	80 COMMON	4 TOE-NAIL OR 2 END DIRECT
STUD TO CAP PLATE:	80 COMMON	4 TOE-NAIL OR 2 END DIRECT
DOUBLE STUDS:	16D COMMON	9" O.C. DIRECT
CORNER STUDS:	16D COMMON	16" O.C. DIRECT
SOLE PLATE TO JOIST OR BLOCKING:	16D COMMON	8" O.C. DIRECT
DOUBLE CAP PLATE:	16D COMMON	16" O.C. DIRECT
CAP PLATE LAPS:	16D COMMON	2 DIRECT
HEADER END TO TRIMMER TO SILL OR GIRDER:	80 COMMON	3" O.C. DIRECT OR 2" O.C. TOE NAIL
APA RATED SHEATHING:	80 COMMON	4" O.C. AT EDGES 8" O.C. OTHER
GYPSUM SHEATHING	GALV. 1/2 GAGE X1 7/8" W/ 7/16" HEAD, DIAMOND POINT	4" O.C. ALONG ALL BEARING POINTS

ROOF & CEILING CONSTRUCTION

- NAILS AND SPIKES SHALL CONFORM TO THE NOMINAL SIZES SPECIFIED IN FEDERAL SPECIFICATIONS FF-N-105B. PNEUMATIC OR ELECTRIC POWERED HAMMERS TYPICALLY UTILIZE LIGHTER GAGE FASTENERS AND NORMALLY REQUIRE ADDITIONAL FASTENERS TO BE INSTALLED. FASTENER SPECIFICATIONS MUST BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO INSTALLATION IF POWERED HAMMERS ARE TO BE USED.
- LAG AND WOOD SCREWS SHALL CONFORM TO ANSII/A306 STANDARD B18.6.1 - 1991.
- ALL FASTENERS USED IN CONTACT WITH PRESERVATIVE PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL. DO NOT MIX STAINLESS STEEL WITH GALVANIZED FASTENERS OR CONNECTORS.
- BORED LEAD HOLES FOR FASTENERS SHALL BE AS FOLLOWS:
 - a. NAIL AND SPIKE LEAD HOLES ARE NOT REQUIRED UNLESS TO PREVENT SPLITTING OF WOOD. IF REQUIRED, LEAD HOLE DIAMETER SHALL NOT EXCEED 75% OF NAIL/SPIKE DIAMETER.
 - b. WOOD SCREWS - LEAD HOLE DIAMETER EQUALS 7/8 OF UNTHREADED SHANK DIAMETER IN CONNECTED WOOD-PART AND 7/8 OF DIAMETER AT ROOT OF THREAD IN WOOD RECEIVING THREAD.
 - c. LAG SCREWS - LEAD HOLE DIAMETER EQUALS SHANK DIAMETER FOR EXTENT OF UNTHREADED SHANK, AND 60% OF SHANK DIAMETER FOR THREADED PORTION OF SHANK.
 - d. THRU BOLTS - LEAD HOLE DIAMETER 1/32" TO 1/16" LARGER THAN NOMINAL BOLT DIAMETER.
- INSERT THREADED SCREW TYPE FASTENERS BY TURNING WITH SCREWDRIVER OR WRENCH. DO NOT DRIVE BY HAMMERS. FACILITATE INSTALLATION BY PLACING SOAP OR OTHER LUBRICANT ON THREADS.
- PROVIDE STANDARD ROUND WASHERS UNDER THE HEADS OF ALL THRU BOLTS AND LAG SCREWS AND UNDER ALL NUTS UNLESS OTHERWISE NOTICED ON THE PLANS. TIGHTEN FASTENERS WITHOUT CRUSHING WOOD FIBERS UNDER WASHERS.

ABBREVIATIONS AND DESIGNATIONS:

ACI	AMERICAN CONCRETE INSTITUTE
ASCE	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ARCH	ARCHITECT
ASTM	AMERICAN SOCIETY OF TESTING MATERIALS
AWS	AMERICAN WELDING SOCIETY
BM	BEAM
BOT	BOTTOM
BTWN	BETWEEN
BY	BOTTOM OF
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL.	COLUMN
CONC	CONCRETE
CL	CENTERLINE
DIAM	DIAMETER
DM	DIMENSION
DWG	DRAWING
ELEV	ELEVATION
EDGS	EDGE OF SLAB
EQ	EQUAL
EXP	EXPANSION
FDN	FOUNDATION
FT	FOOT OR FEET
FTG	FOOTING
G	GAGE
GALV	GALVANIZED
HORIZ	HORIZONTAL
INT	INTERIOR
INT	INTERIOR
K	KIPS
L	STEEL ANGLE
LBS	POUNDS
LVL	LAMINATED VENEER LUMBER
MAX	MAXIMUM
MIN	MINIMUM
NDS	NATIONAL DESIGN SPECIFICATION
NO	NUMBER
NTS	NOT TO SCALE
OC	ON CENTER
PAF	POWER ACTUATED FASTENER
PERP	PERPENDICULAR
PLF	POUNDS PER LINEAR FOOT
PSF	POUNDS PER SQUARE FOOT
QTY	QUANTITY
SECT	SECTION
SM	SIMILAR
T	TOP OF
UON	UNLESS OTHERWISE NOTED
VE	VERIFY IN FIELD
W/W	WELDED WIRE FABRIC
Ø	DIAMETER



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Scranton, PA 18608

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EAST GREENBUSH, NY 12061
P: (518) 366-9980
www.preston-eng.com



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138 S. Main St., Wilkes Barre, PA 19701, 570-922-4141, www.mra-arch.com



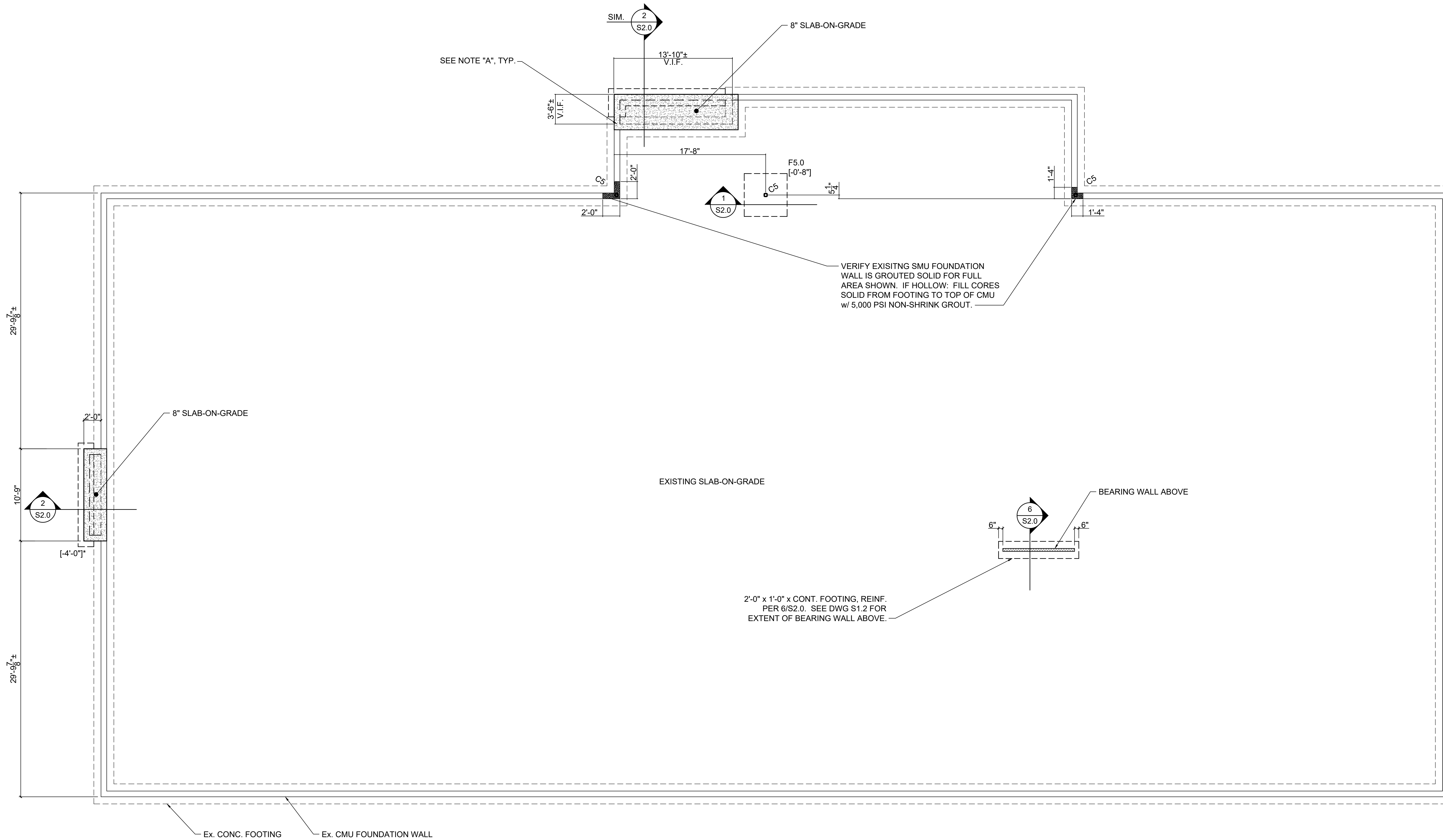
PROJECT #220646
GARDEN, EXTY / GARDEN INT.

ONVO TRAVEL PLAZA
ONVO/BURGER KING
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GENERAL NOTES & SPECIFICATIONS

S1.0

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FOUNDATION PLAN NOTES

TOP OF SLAB ELEVATION SHALL MATCH TOP OF EXISTING SLAB ELEV. 0'-0" (DATUM ELEVATION)

VESTIBULE SLAB: 8" SLAB-ON-GRADE REINF. AS PER 2/S2.0

SAWCUT, REMOVE AND REPLACE EXISTING SLAB-ON-GRADE AS NEEDED TO INSTALL SUB-SLAB PIPING, CONDUITS, ETC. MATCH THICKNESS OF EXISTING SLAB, AND REINFORCE w/ 6x6 - W2.0 x W2.0 W.W.F. SEE 5/S2.0.

[-X'-XX"] INDICATES TOP OF FOOTING ELEVATION.
 * INDICATES TOP OF FOOTING ELEVATION IS APPROXIMATE. SET BOTTOM OF FOOTING ELEVATION TO MATCH BOTTOM OF EXISTING FOOTING ELEVATION.

ALL FOOTINGS SHALL BE 3'-6" MIN. BELOW EXT. GRADE ELEVATION FOR FROST PROTECTION

F5.0 = 5'-0" x 5'-0" x 1'-4" CONC. FOOTING REINF. w/ (4) - #4 BOTTOM EACH WAY

C5 INDICATES HSS5x5x5/16 COLUMN w/ 3/4" BASE PLATE WELDED TO COLUMN w/ 1/4" FILLET WELD ALL AROUND:
 • COLUMNS AT CORNERS: "L"-SHAPED 12" x 3/4" x 1'-0" BASE PLATE w/ (2) - 3/4" Ø ANCHOR RODS. (REMOVE 6"x6" AREA AT BASE PLATE CORNER)
 • INTERIOR COLUMN: 12" x 3/4" x 1'-0" BASE PLATE w/ (4) - 3/4" Ø ANCHOR RODS.

COORDINATE ALL SLEEVES, PENETRATIONS AND INSERTS IN CONCRETE WALLS AND SLABS WITH ARCHITECTURAL, CIVIL AND MECHANICAL DRAWINGS.

AT NEW DOOR OPENINGS, SAWCUT AND REMOVE TOP COURSE OF EXISTING CMU FOUNDATION WALL, SIMILAR TO 2/S2.0. REPLACE w/ 8" NEW CONCRETE w/ (2) #4 HORIZ. DRILL AND EMBED ALTERNATING BAR ENDS INTO ADJACENT FOUNDATION WALL TO REMAIN. DRILL AND EMBED 5" w/ HILTI "HIT HY 270". PROVIDE SCREEN TUBE INSERTS IF WALL IS HOLLOW

SEE DWG S1.0 FOR GENERAL NOTES AND SPECIFICATIONS

NOTE "A":

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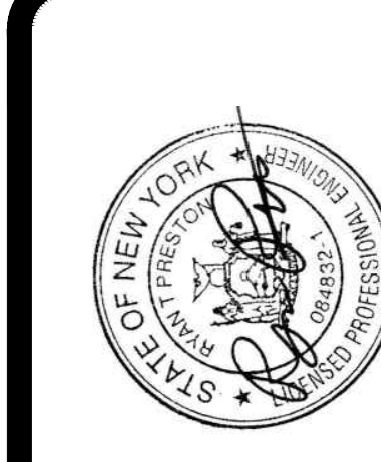


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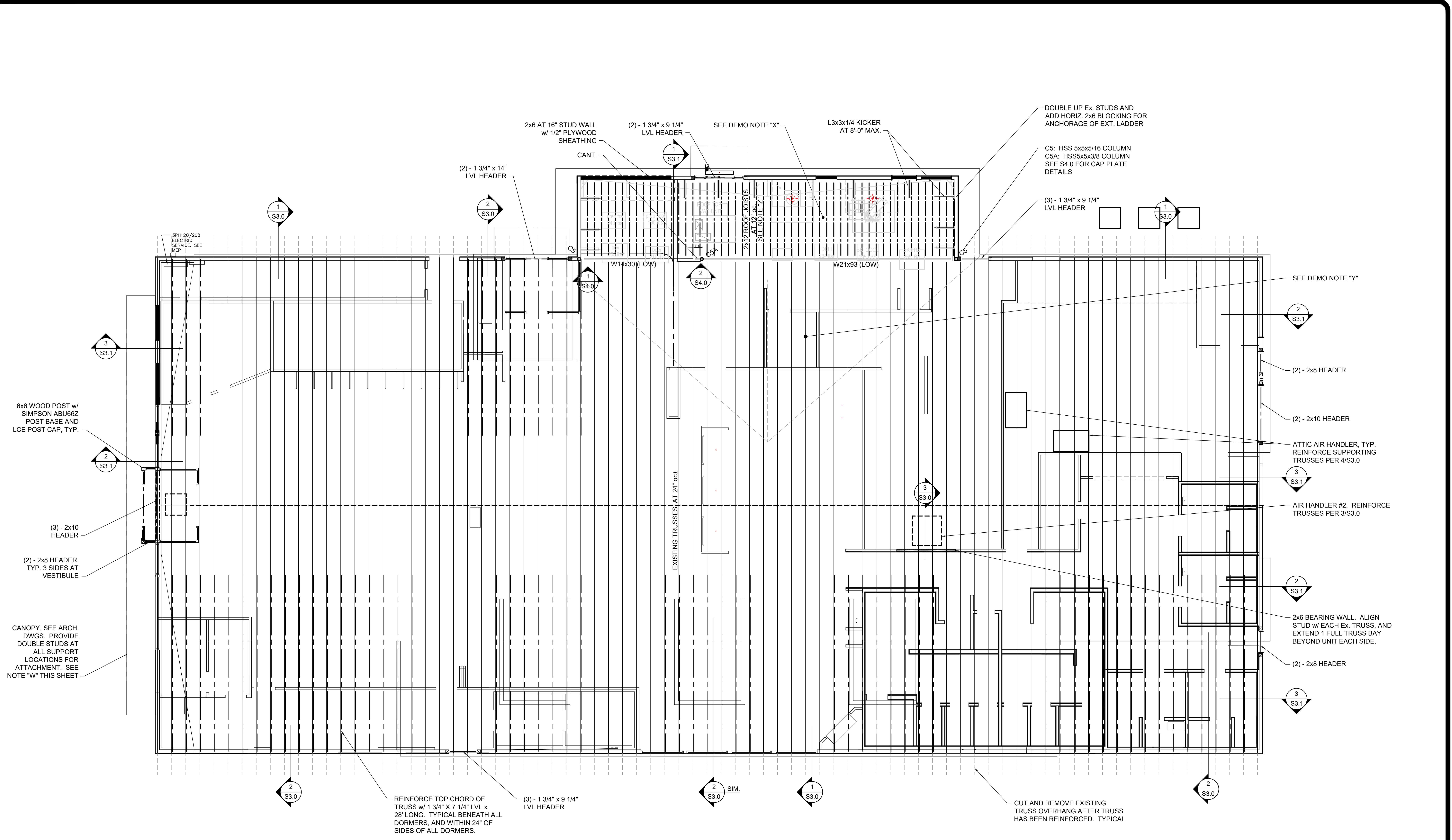
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 FOUNDATION PLAN



ROOF FRAMING PLAN NOTES

3/16" = 1'-0"

ROOF SHEATHING: 3/4" MIN. PLYWOOD
WALL SHEATHING: 1/2" MIN. PLYWOOD

ALL EXTERIOR FRAMING TO BE P.T.
EXTERIOR PSL BEAMS TO BE WOLMANIZED

ALL FASTENERS/ HANGERS/CAPS, ETC. TO BE GALVANIZED

REINFORCE TRUSSES PRIOR TO CONSTRUCTING OVERBUILDS.

AT DORMER OVERBUILDS, REMOVE EXISTING SHINGLES. DO NOT REMOVE EXISTING PLYWOOD EXCEPT AS NEEDED FOR PARAPET CONSTRUCTION

ALL NEW OVERBUILD RAFTERS/JOISTS/STUDS SHALL ALIGN W/ AN EXISTING TRUSS OR ROOF FRAMING MEMBER

AT HEADERS WHERE NO POST IS INDICATED, PROVIDE (2) JACK + (2) KING STUD POST IN SIDE WALLS, (1) JACK + (2) KING STUD POST IN END WALLS

SEE DWG S1.0 FOR GENERAL NOTES AND SPECIFICATIONS

NOTE "W":
FASTEN UPPER CANOPY SUPPORT BRACKET TO SOLID 4x6 BLOCKING BETWEEN DOUBLE STUDS. FASTEN BLOCKING TO DOUBLE STUDS w/ SIMPSON (2) LS50 FRAMING ANGLES. AND FASTEN DOUBLE STUDS TO TOP/BOTTOM PLATE w/ (2) SIMPSON LS50, ONE EACH SIDE

DEMO NOTE "X":
BRACE WALLS AND REMOVE EXISTING ROOF TRUSSES AND ROOF SHEATHING

DEMO NOTE "Y":
BRACE WALLS AND REMOVE EXISTING OVERBUILD ROOF FRAMING AND OVERBUILD ROOF SHEATHING. INFILL OPENINGS IN EXISTING ROOF SHEATHING TO REMAIN w/ 3/4" PLYWOOD

NOTE "Z":
PROVIDE DOUBLE 2x12 JOISTS AT THE FOLLOWING LOCATIONS:

- BENEATH EACH CORNER OF THE ROOF TOP HVAC UNIT
- AT POINTS OF ATTACHMENT FOR THE GREASE HOOD
- AT ANY LOCATIONS SUPPORTING TWO OR MORE OTHER PIECES OF EQUIPMENT (e.g. CONDENSERS, FANS, ETC.)
- AT ANGLE KICKER LOCATIONS

3PH120/208
ELECTRIC
SERVICE. SEE
MEP

6x6 WOOD POST w/
SIMPSON ABU66Z
POST BASE AND
LCE POST CAP. TYP.

(3) - 2x10
HEADER

(2) - 2x8 HEADER,
TYP. 3 SIDES AT
VESTIBULE

CANOPY, SEE ARCH.
DWGS. PROVIDE
DOUBLE STUDS AT
ALL SUPPORT
LOCATIONS FOR
ATTACHMENT. SEE
NOTE "W" THIS SHEET

REINFORCE TOP CHORD OF
TRUSS w/ 1 3/4" X 7 1/4" LVL x
28' LONG. TYPICAL BENEATH ALL
DORMERS, AND WITHIN 24" OF
SIDES OF ALL DORMERS.

(3) - 1 3/4" x 9 1/4"
LVL HEADER

CUT AND REMOVE EXISTING
TRUSS OVERHANG AFTER TRUSS
HAS BEEN REINFORCED. TYPICAL

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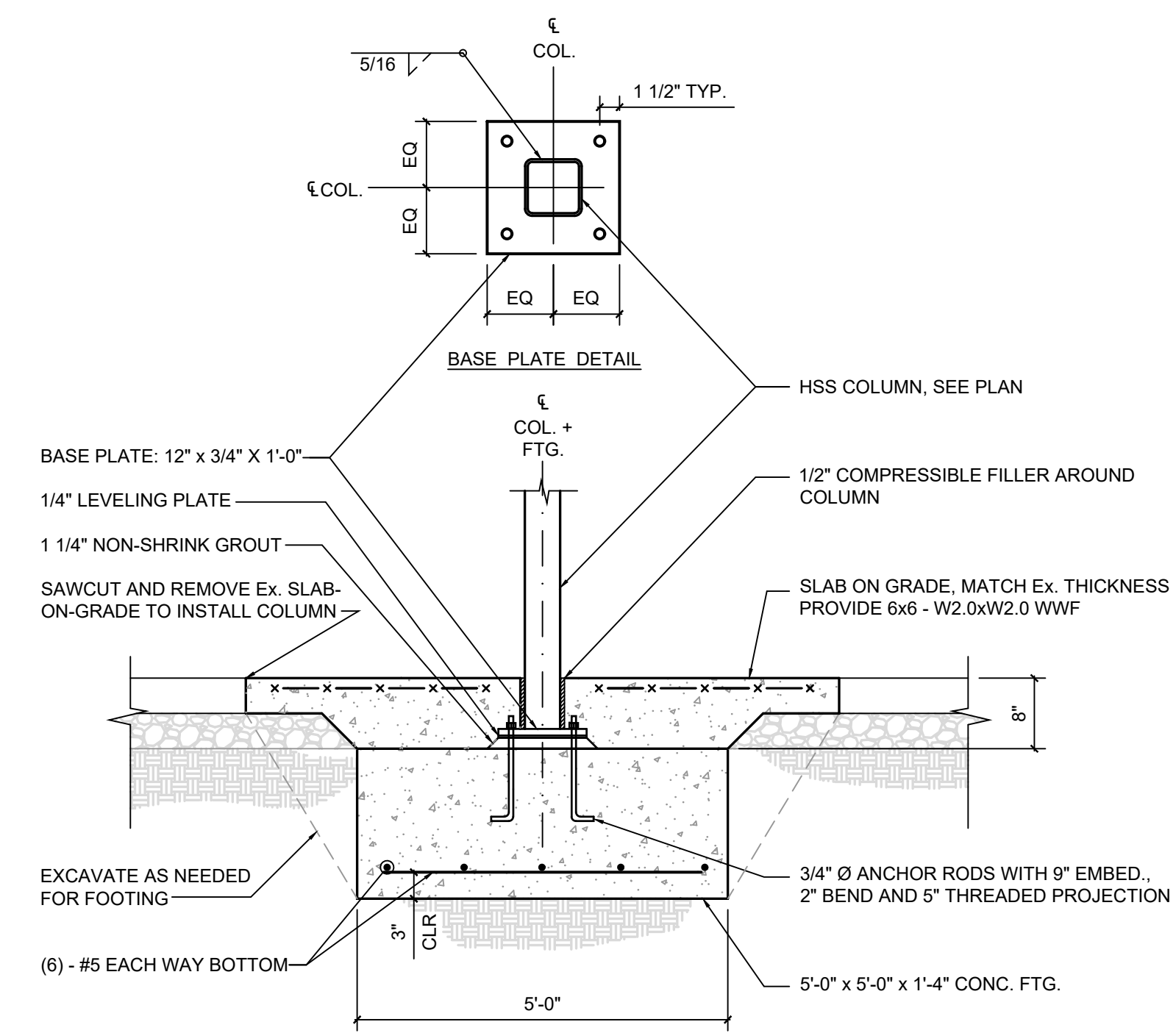
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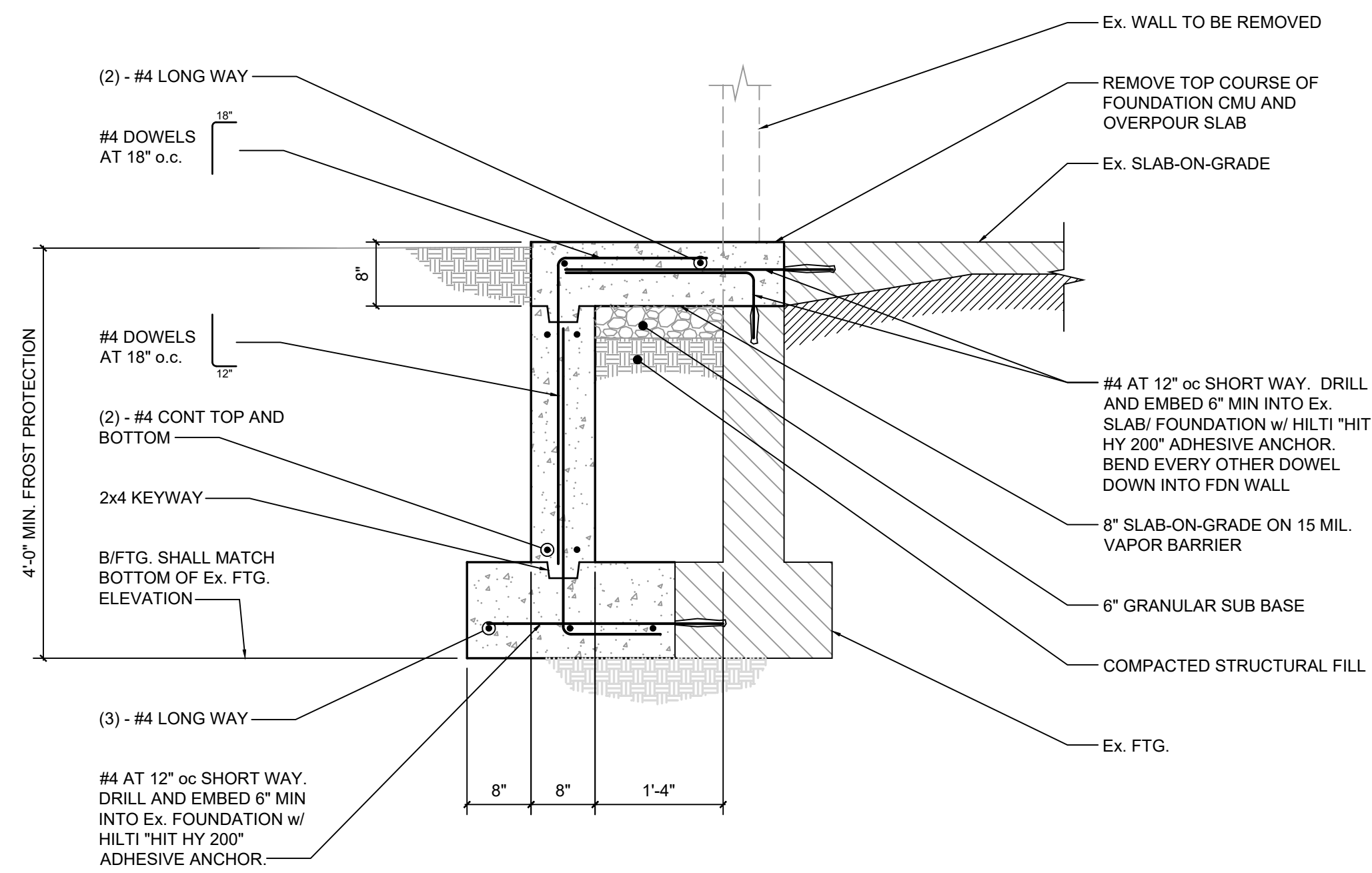
ROOF FRAMING PLAN

S1.2

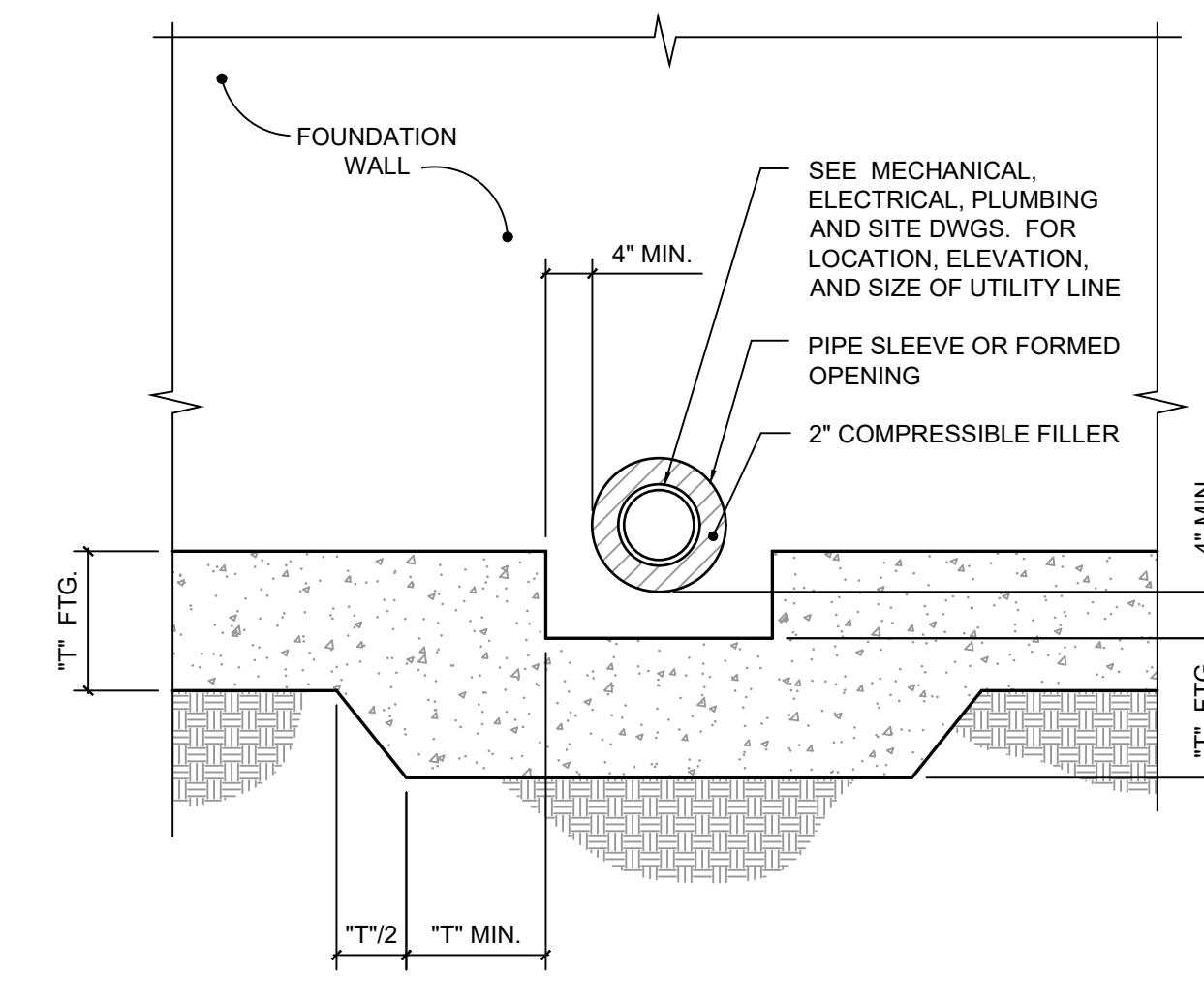


1 SECTION
3/4" = 1'-0"

NOTE:
AT THE CONTRACTOR'S OPTION,
ANCHOR RODS MAY BE DRILLED AND
EMBEDDED 6" MIN. INTO CONCRETE
WITH HILTI "HIT HY 200" ADHESIVE
ANCHORING SYSTEM.

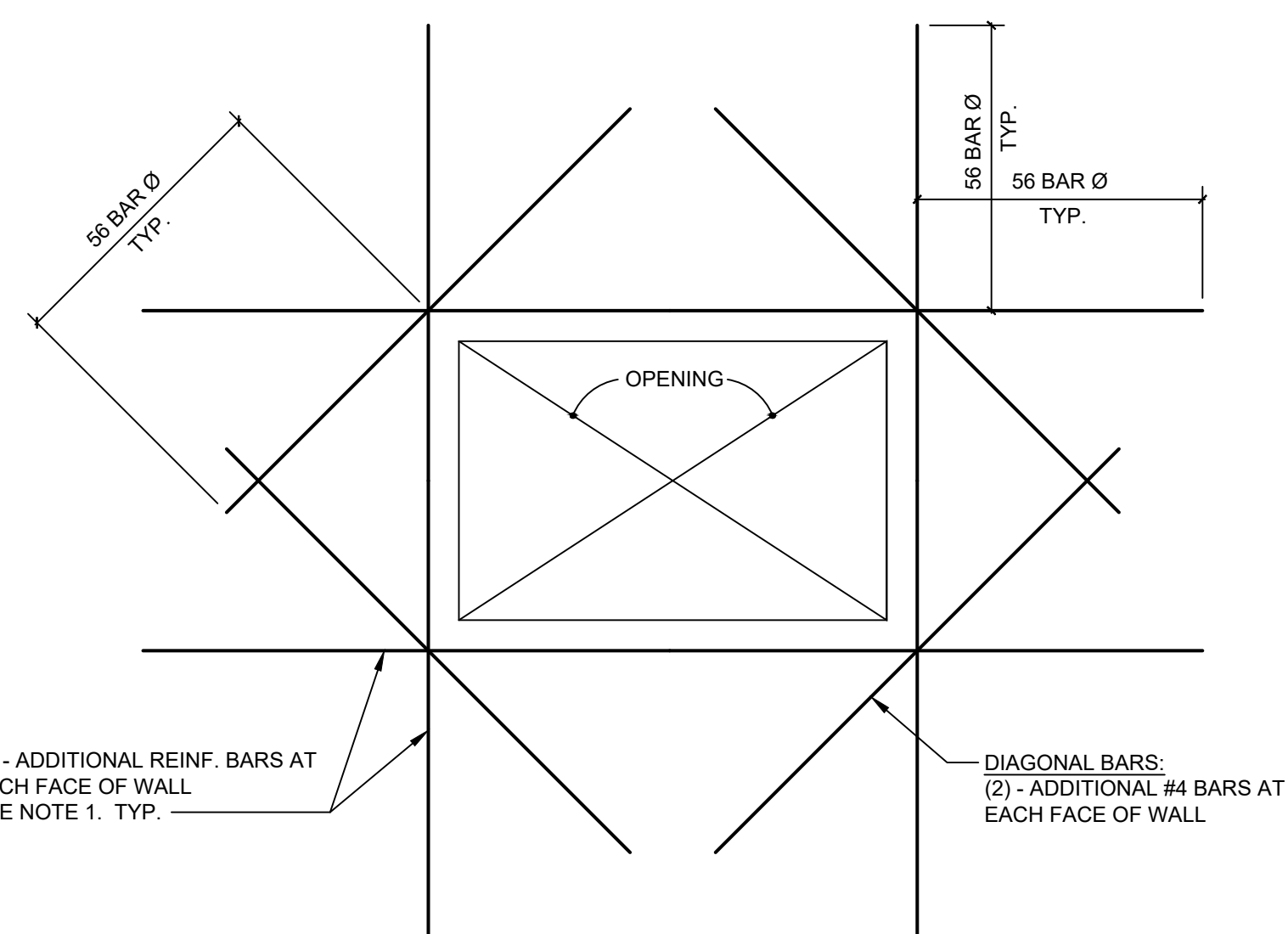


2 SECTION
3/4" = 1'-0"



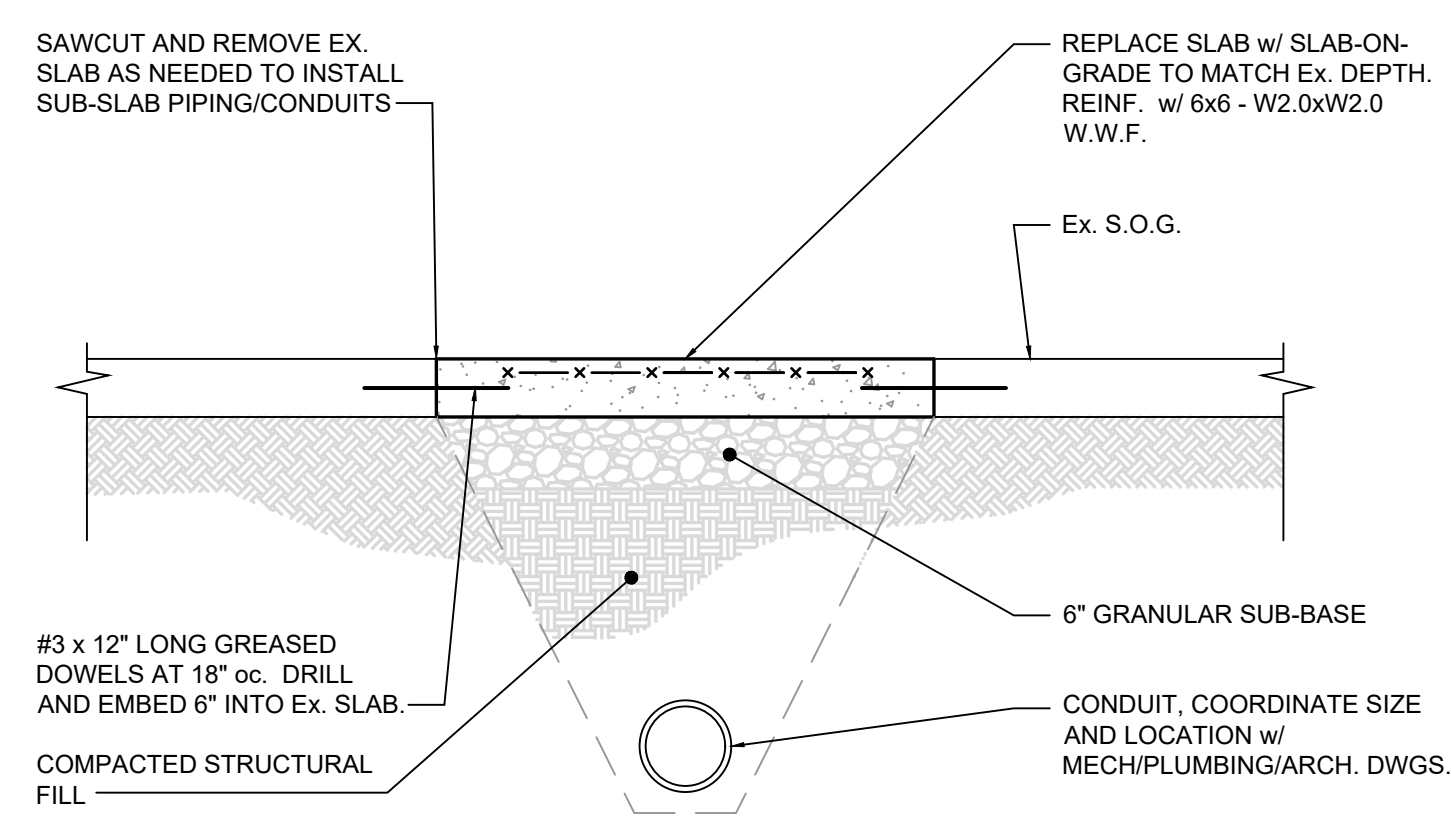
3 TYP. STEP FTG. DET. AT PIPE PENETRATION
NO SCALE

NOTES:
FOR ADDITIONAL WALL REINFORCING, SEE
"TYPICAL SLAB/WALL PENETRATION REINFORCING DETAIL".

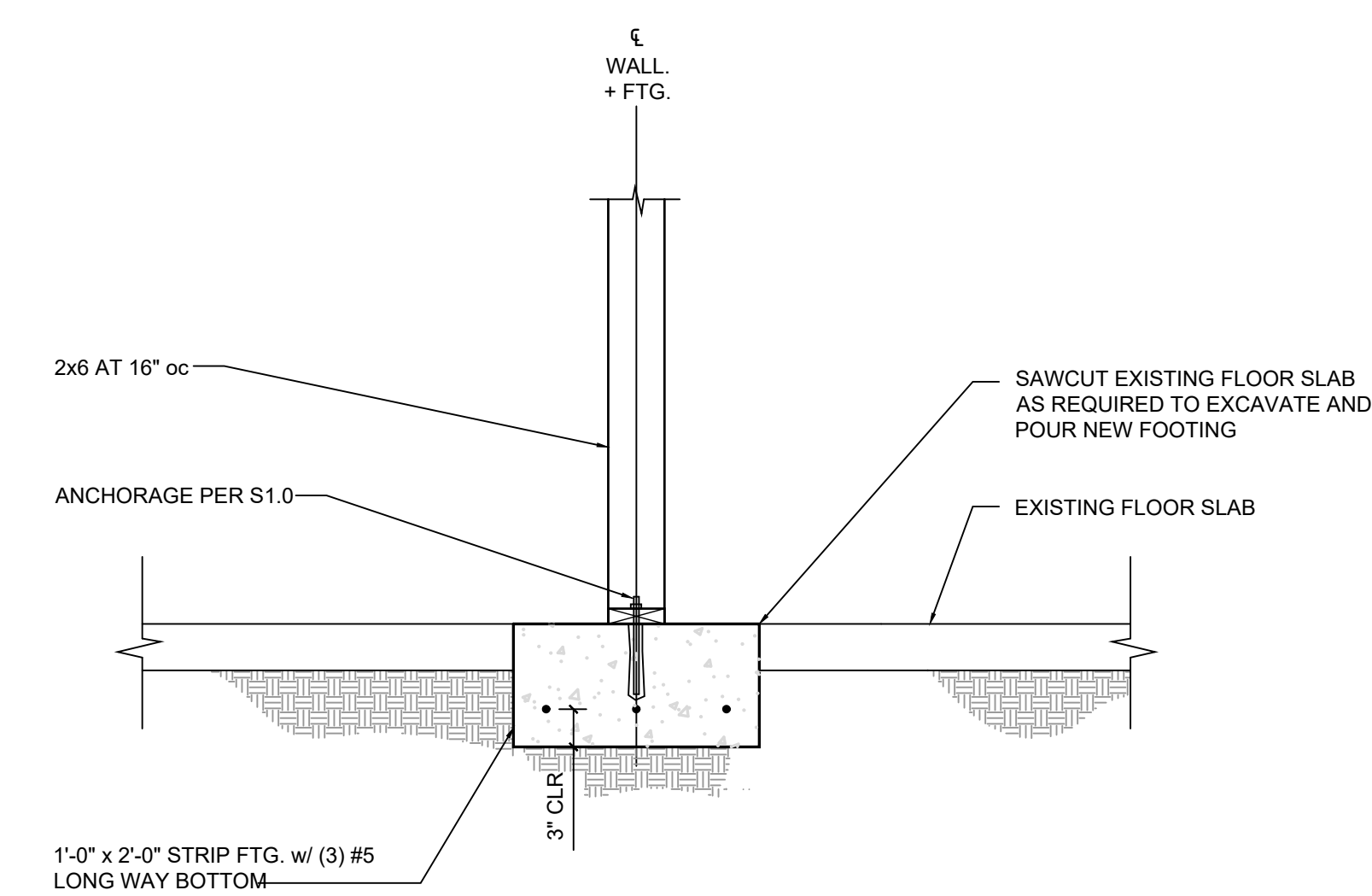


4 TYPICAL WALL OPENING REINF.
NO SCALE

NOTES:
1. AT WALLS, ADDITIONAL REINFORCING SIZE SHALL MATCH
HORIZONTAL AND VERTICAL REINFORCING
2. THIS DETAIL APPLIES AT ALL OPENINGS 1'-0" X 1'-0" AND LARGER.
DETAIL IS SIMILAR AT CIRCULAR OPENINGS 1'-0" Ø AND LARGER.
3. COORDINATE ALL OPENING SIZES AND LOCATIONS WITH
ARCHITECTURAL AND MECHANICAL DRAWINGS.



5 TYP. SLAB REPLACEMENT DETAIL
3/4" = 1'-0"



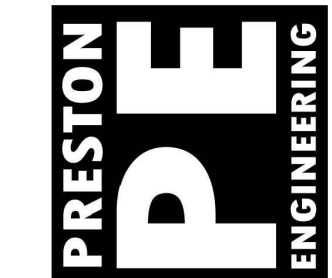
6 TYP. STRIP FTG. SECTION
3/4" = 1'-0"

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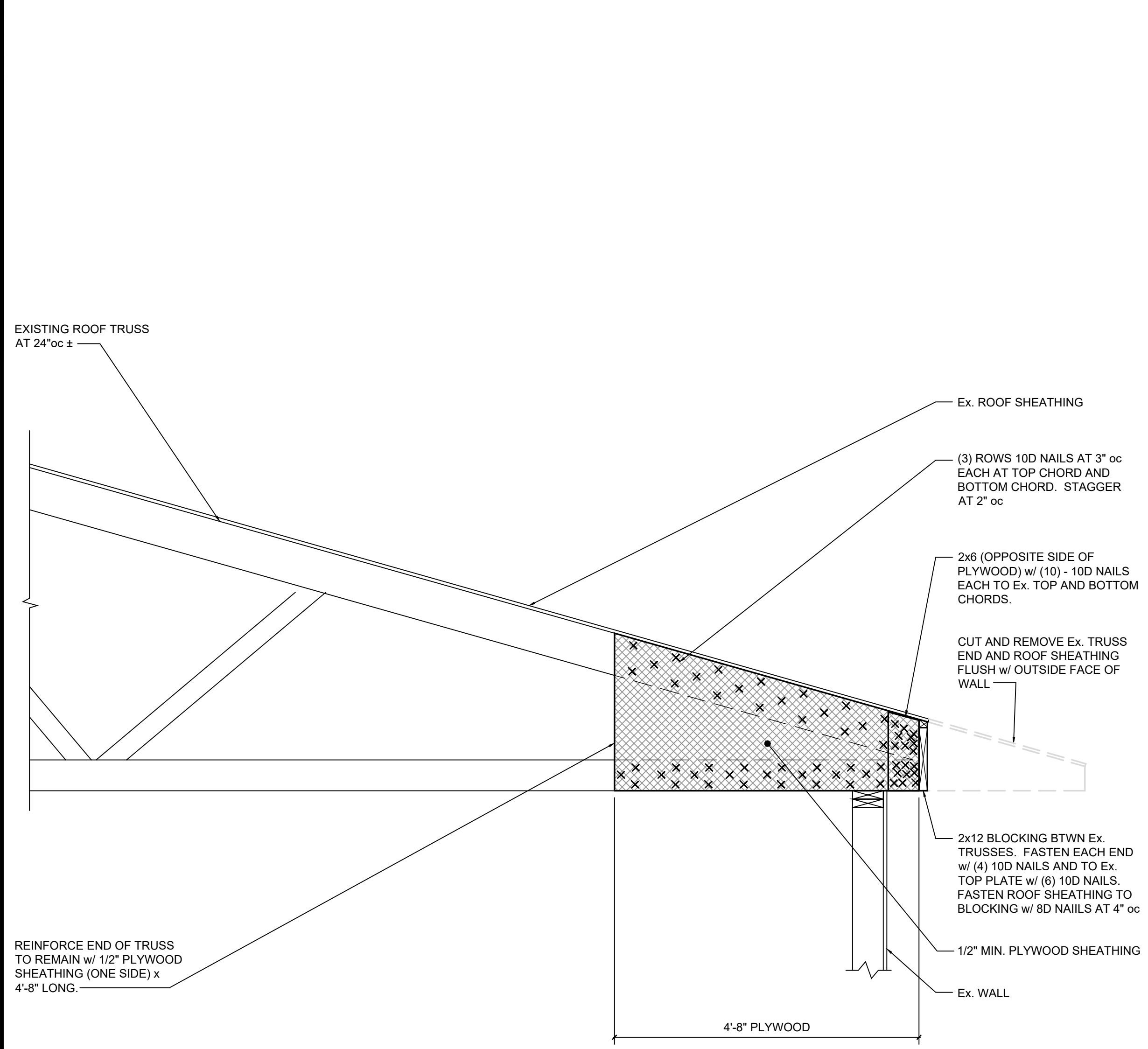
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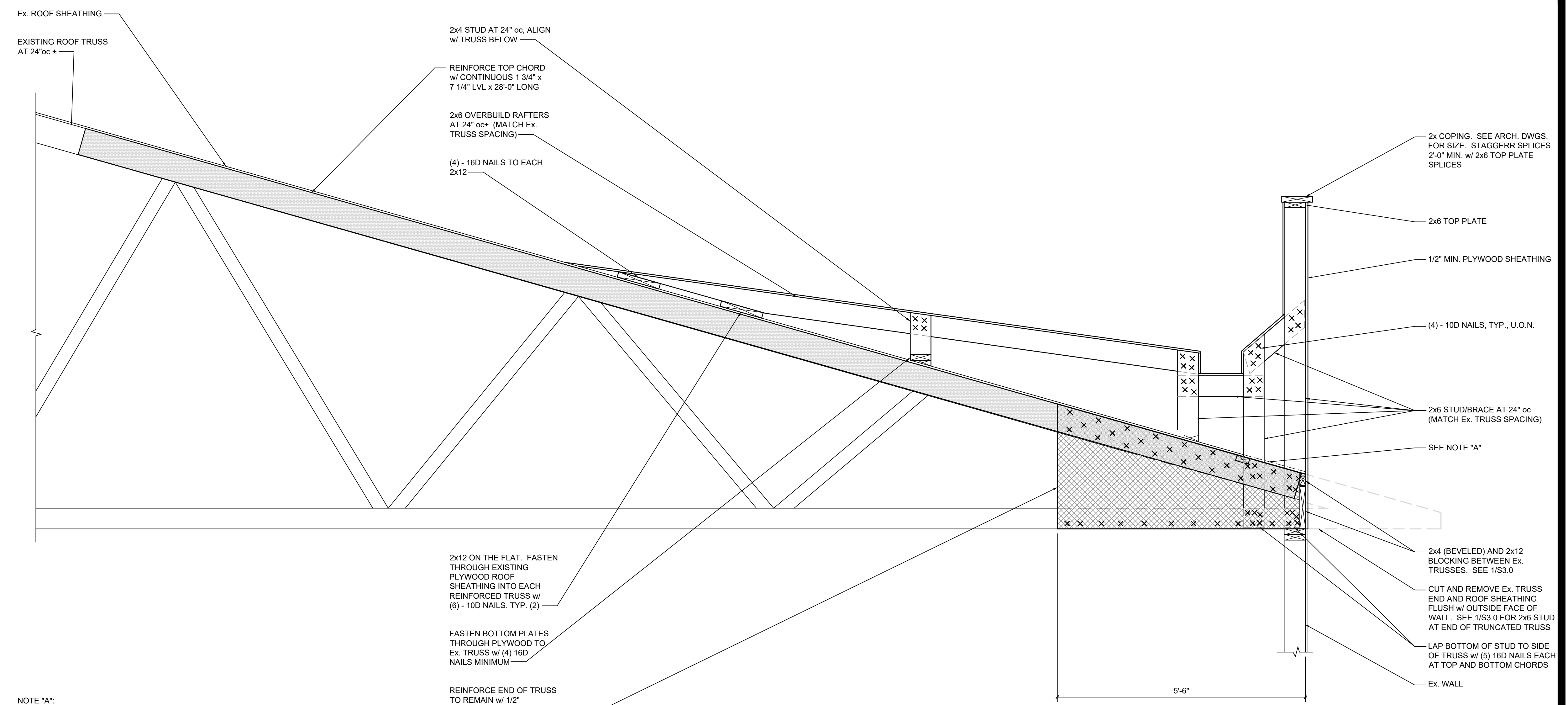
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FRAMING SECTIONS AND DETAILS

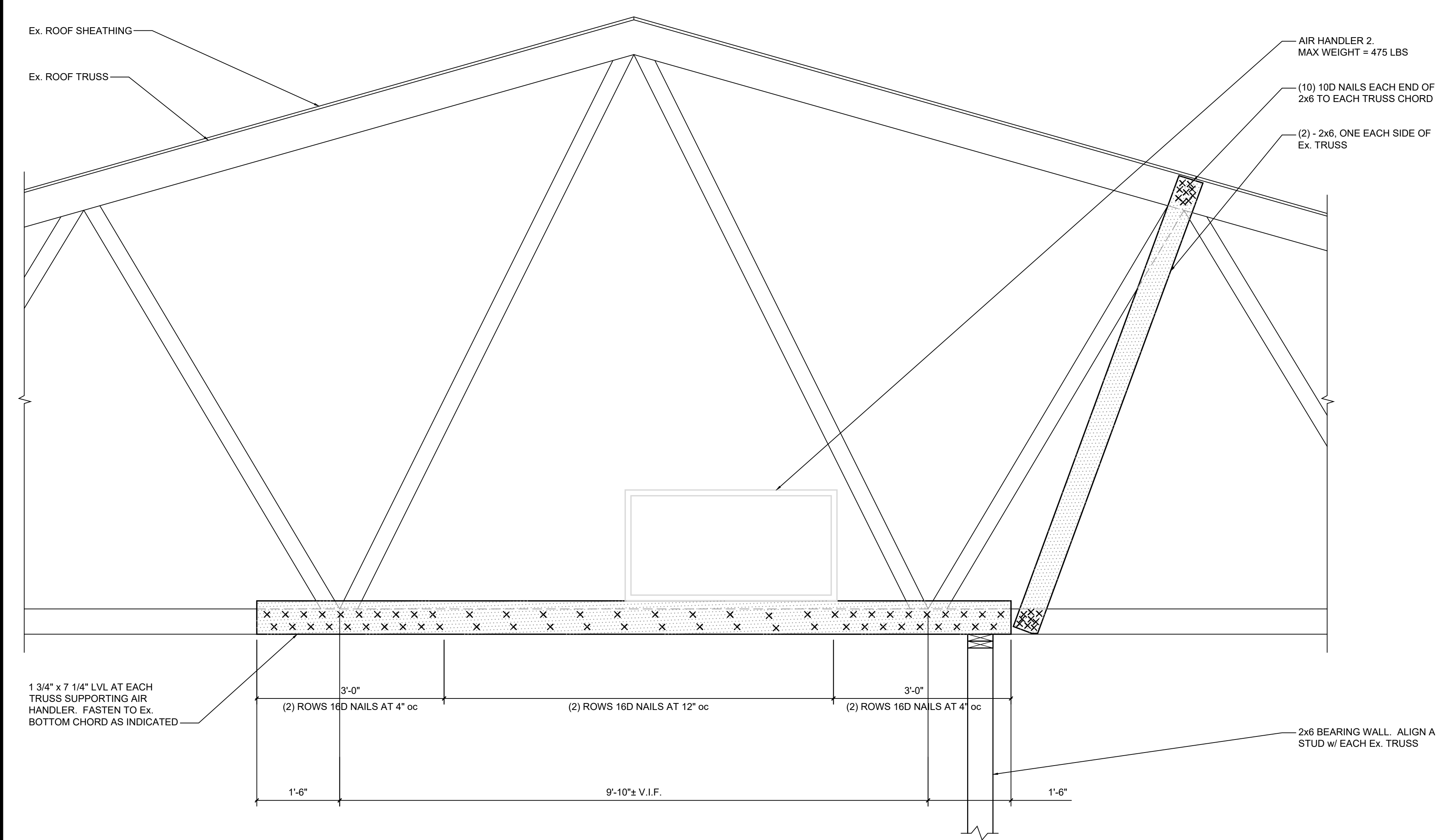
S2.0



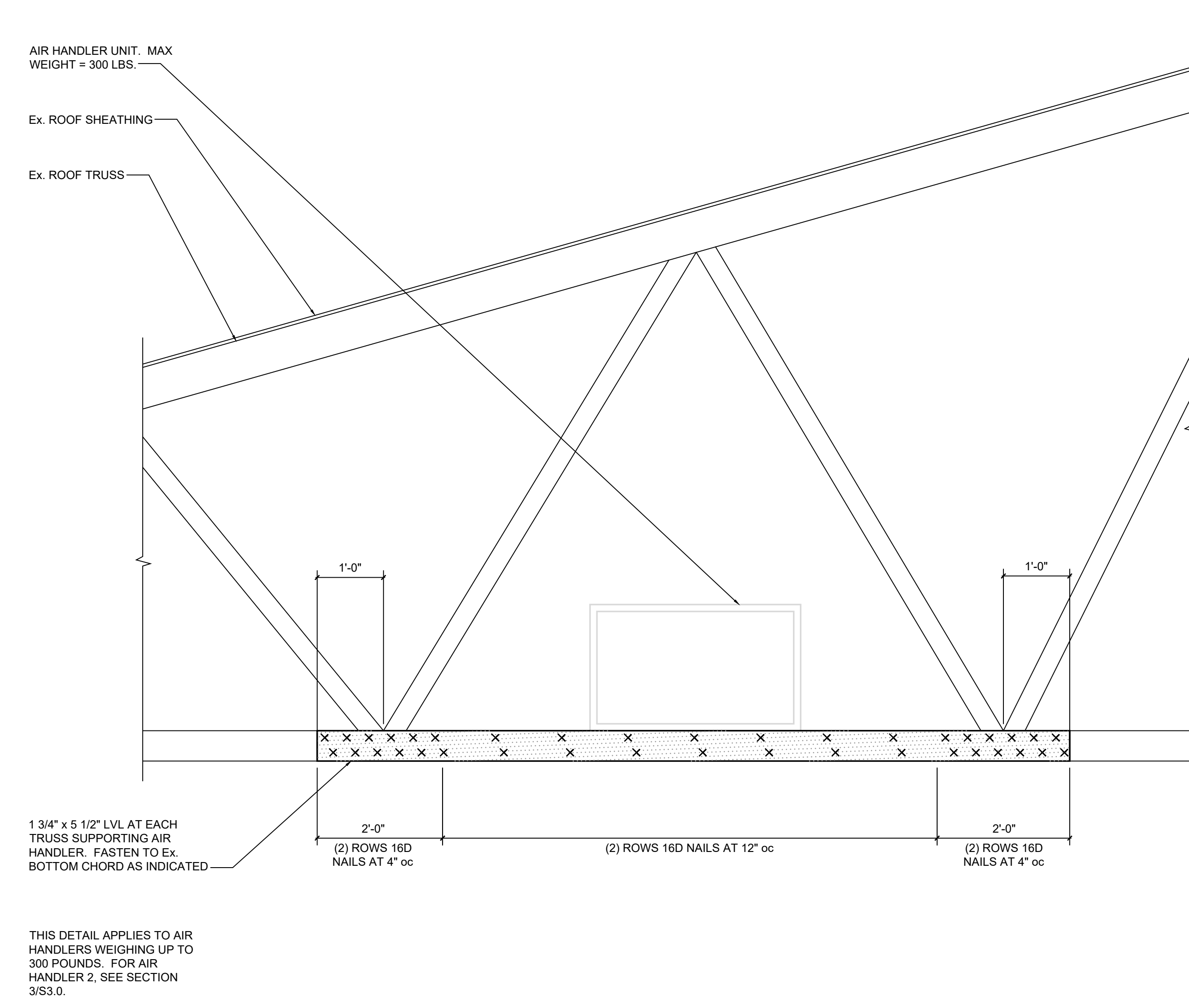
1 SECTION
3/4" = 1'-0"



2 SECTION
3/4" = 1'-0"



3 SECTION AT AIR HANDLER 2
3/4" = 1'-0"



4 TYP. SECTION AT AIR HANDLERS UP TO 300 LBS
3/4" = 1'-0"

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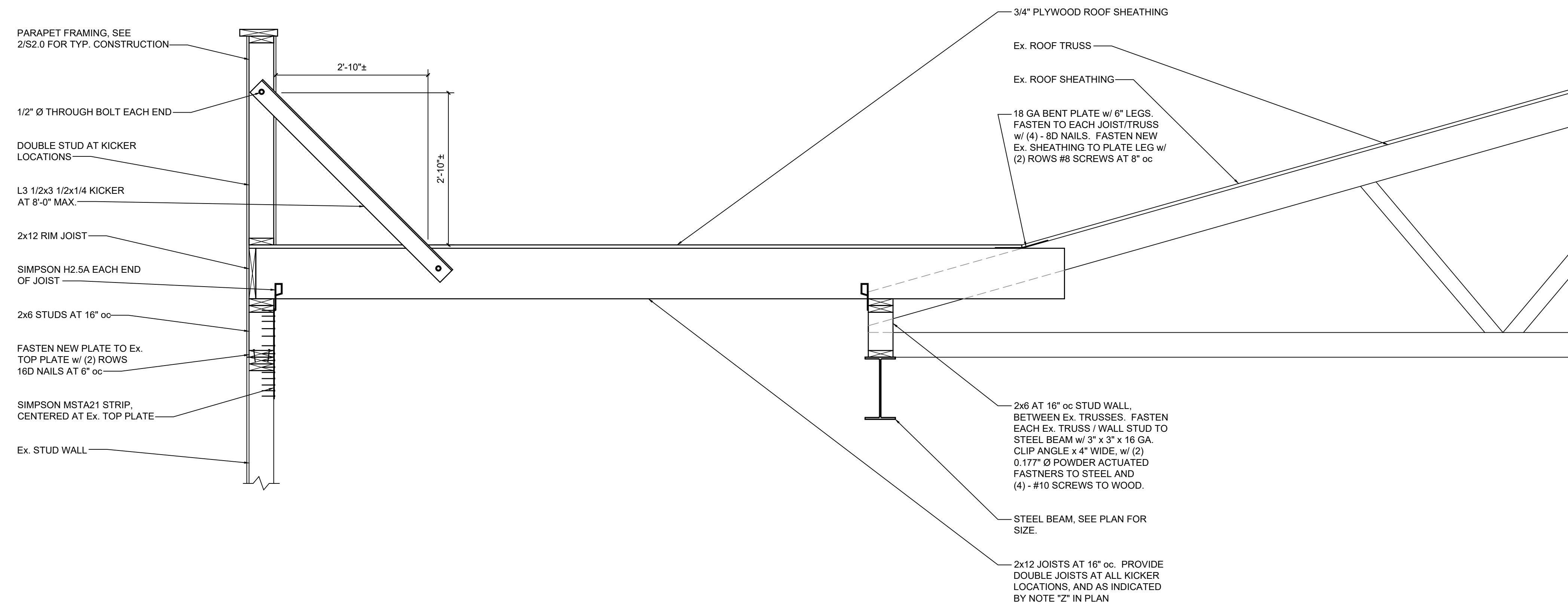
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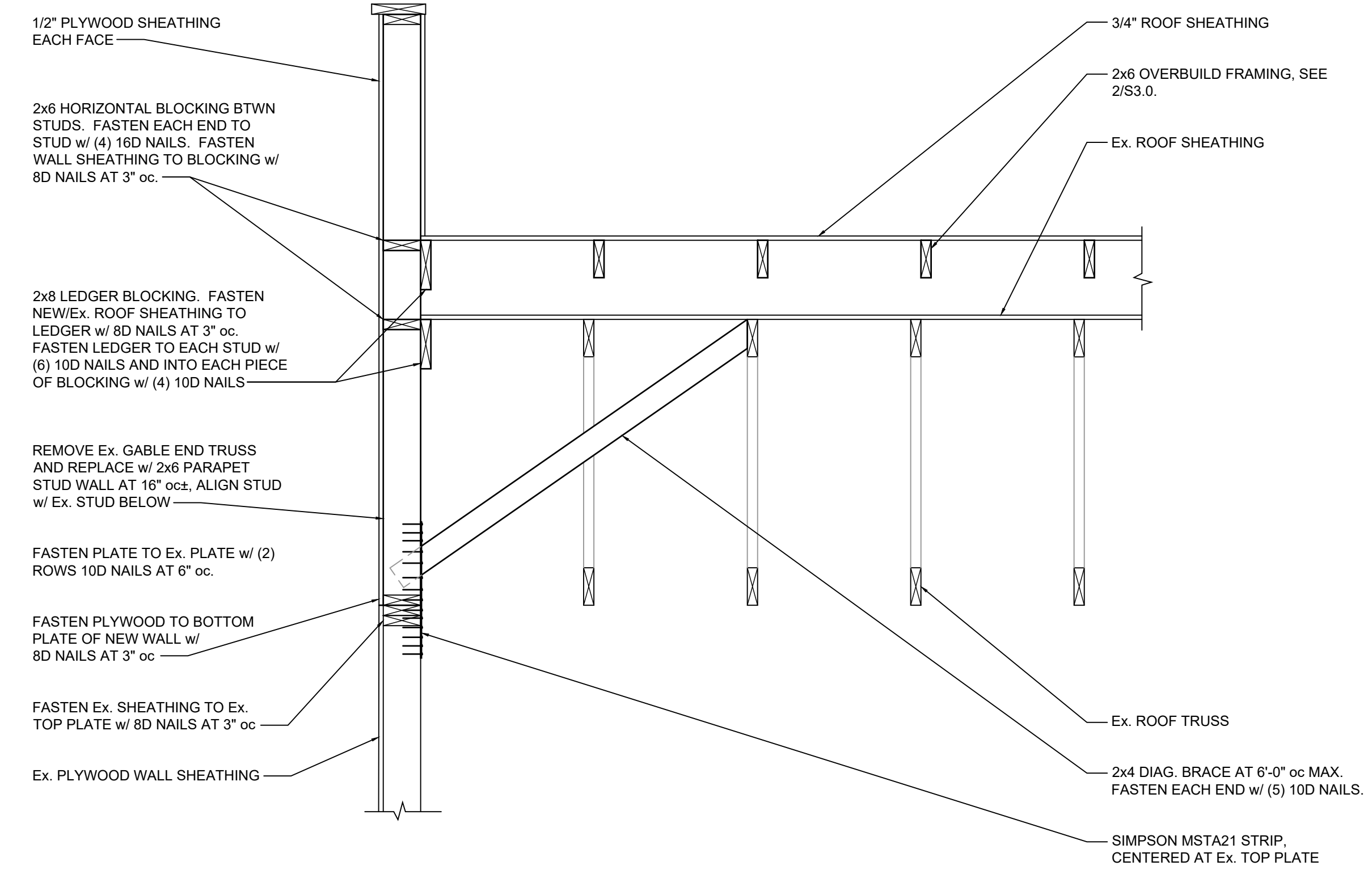
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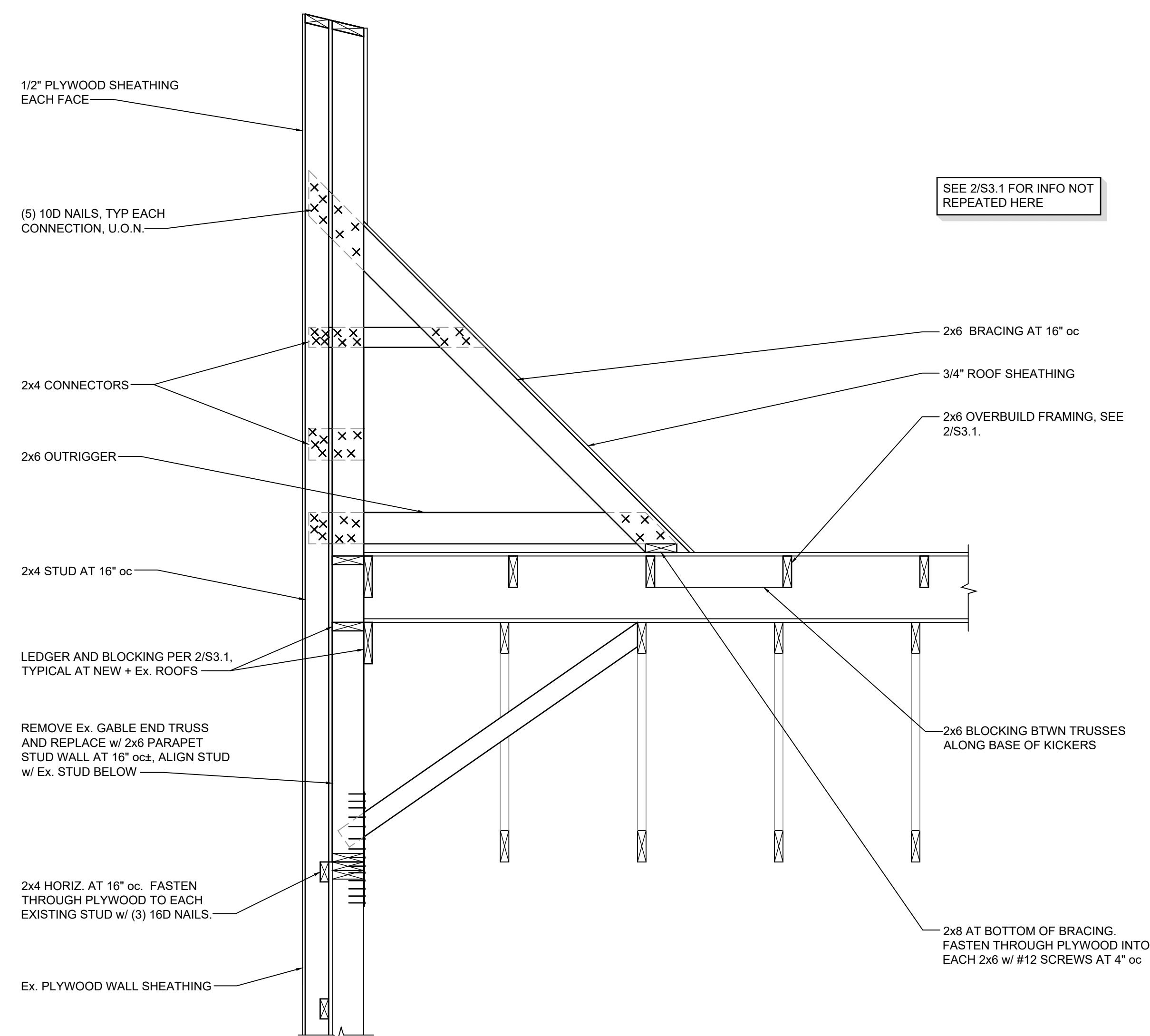
S3.0



1 SECTION
3/4" = 1'-0"



2 SECTION
3/4" = 1'-0"



3 SECTION
3/4" = 1'-0"

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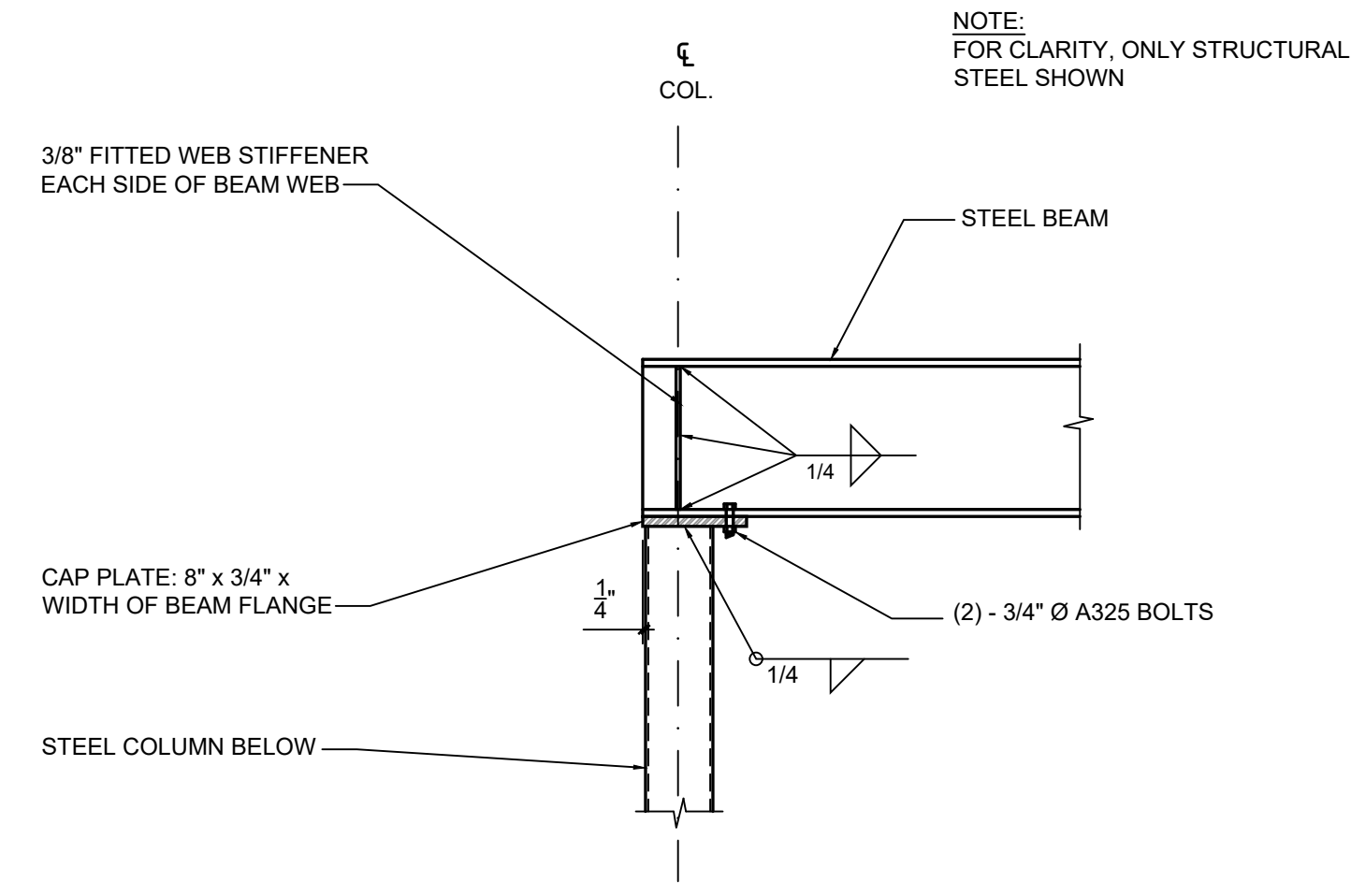
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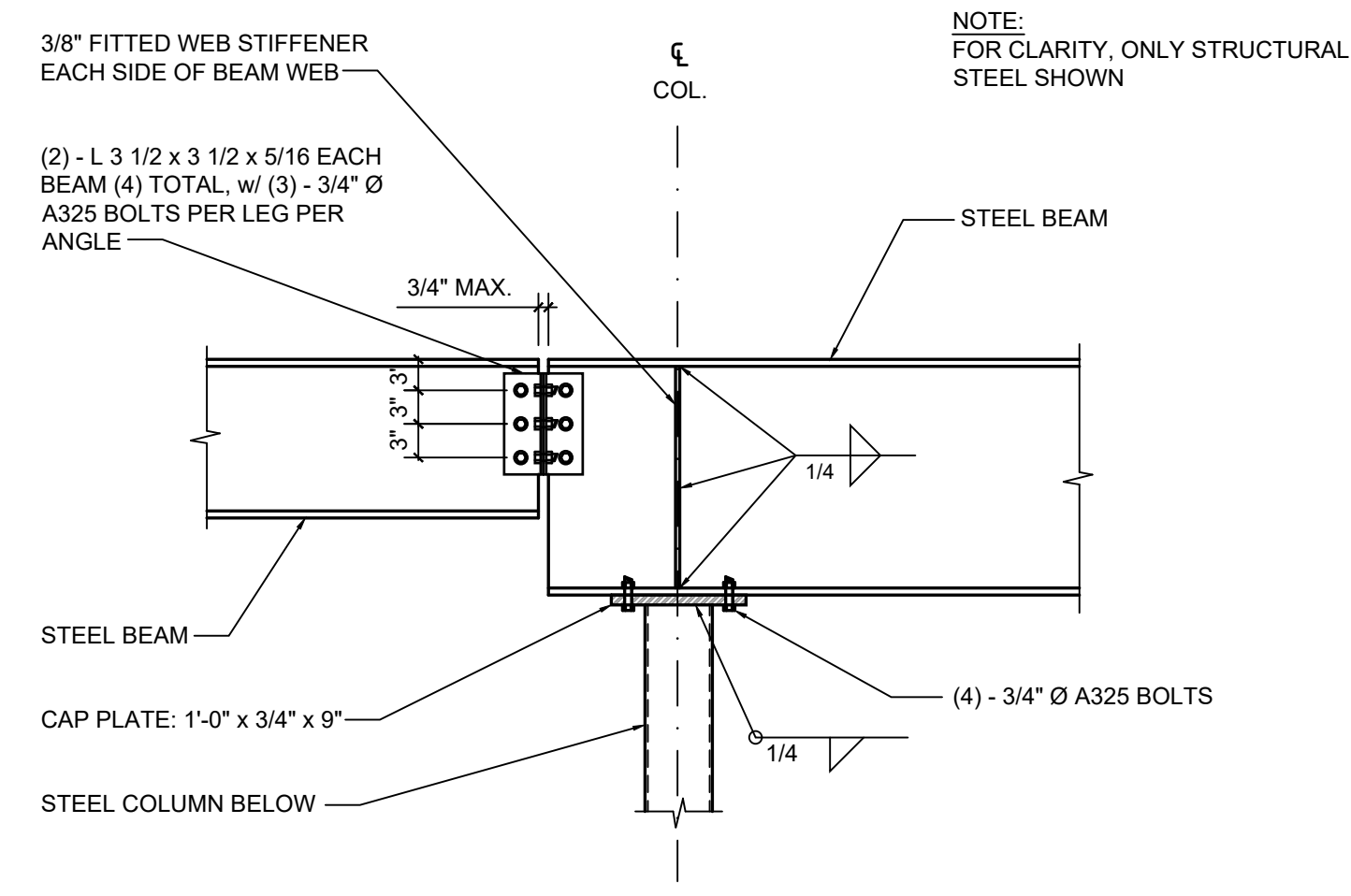
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 FRAMING SECTIONS AND DETAILS



1 TYPICAL STEEL BEAM TO COLUMN DETAIL
3/4" = 1'-0"



2 TYPICAL STEEL BEAM CANT. OVER COLUMN DETAIL
3/4" = 1'-0"

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STEEL FRAMING SECTIONS AND DETAILS
54.0